

Assessing the usage and level-of-service of pedestrian facilities in train stations: A Swiss case study

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October 8, 2015

Report TRANSP-OR 151008
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Abstract

A framework for assessing the usage and level-of-service of rail access facilities is presented. It consists of two parts. A dynamic demand estimator allows to obtain time-dependent origin-destination flows within pedestrian facilities. Using that demand, a traffic assignment model describes the propagation of pedestrians through the station, providing an estimate of prevalent traffic conditions in terms of flow, travel times, speed and density. The framework is discussed at the example of Lausanne railway station. For this train station, a rich set of data sources including travel surveys, pedestrian counts and trajectories has been collected in collaboration with the Swiss Federal Railways. Results show a good performance of the framework. Moreover, to underline its practical applicability, a six-step planning guideline is presented that can be used to design and optimize rail access facilities for new or existing train stations.

1 Introduction

Passenger railway systems around the world are undergoing a significant growth. In the last decade, the number of transported passengers in Europe and North America has increased by about 3% annually (Puentes et al., 2013; Kasparick, 2010; Amacker, 2012), while in Asia even two-digit growth rates are common (Chung, 2012; LTA Singapore, 2012). Partially in response to that growth, and partially inducing it, the frequency and capacity of trains have been continuously expanded (Kallas, 2014).

In the context of that expansion, rail access facilities have largely been neglected (Schneider, 2012). Rail access facilities include pedestrian walkways, waiting areas or platforms, and in the broader sense all pedestrian infrastructures within a train station. Today, these facilities are gaining attention as pedestrian congestion is becoming a common phenomenon (Ganansia et al., 2014; Hermant, 2012), waiting space on platforms and in station halls is getting scarce (Hoogendoorn and Daamen, 2004), expectations in terms of comfort and shopping opportunities are growing (Nio, 2012), or safety regulations are violated (Buchmüller and Weidmann, 2008).

To optimize the design and operation of rail access facilities, there is a general need to better understand the usage of railway stations by pedes-

trians (Parkinson and Fisher, 1996). Such knowledge is essential for the adequate dimensioning of infrastructures, such as the width of an underground walkway or the area of a platform. It is also beneficial for an efficient operation of a train station. For instance, should trains with a particularly large ridership be served only by certain platforms, and should the simultaneous arrival of large trains be avoided? How long should transfer times be to allow for sufficient time to reach a connecting train? Which are the optimal walking routes during normal operation and in case of extreme events? Where should sales and service points be located to attract a maximum of walk-in customers?

A way of addressing such questions is by developing a quantitative, spatio-temporal understanding of pedestrian flows. This information can then be used to tackle the mentioned problems, which are often subject to financial, operational, political and legal constraints.

In this article, we present a modeling framework that provides an estimate of pedestrian origin-destination (OD) demand and is able to assess the level-of-service (LOS) of an infrastructure. Figure 1 provides a graphical representation of that framework, considering various data sources, a demand estimator and a traffic assignment model. These elements are discussed one by one in the subsequent sections. In the last section, they are set in a general context, enriched by practical guidelines.

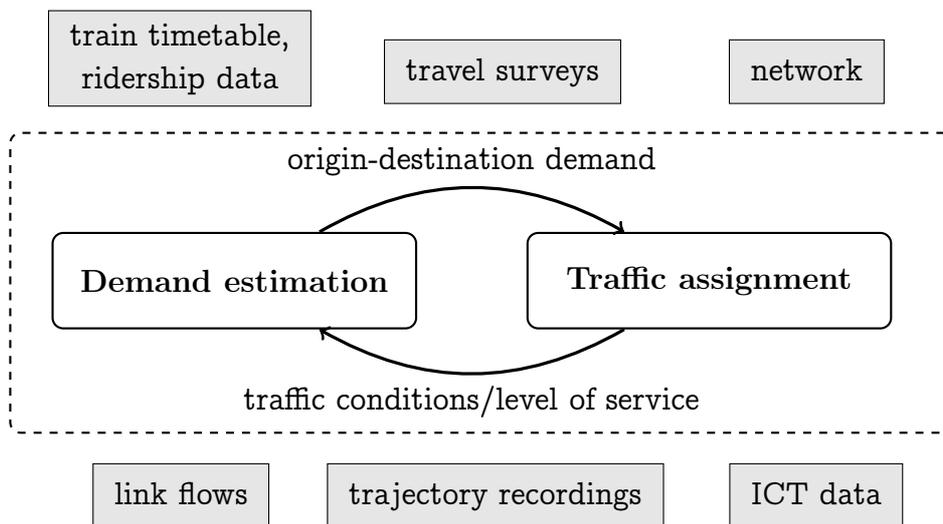


Figure 1: Framework for estimating the usage and level-of-service of rail access facilities.

Throughout this work, reference is made to the railway station of Lausanne, which we have studied together with the Swiss Federal Railways (SBB) between 2011 and 2015. Lausanne railway station has reached capacity in the year 2010, and a doubling of passenger demand is expected by 2030. About € 450,000 have been invested in a pedestrian tracking system to monitor and understand pedestrian movements on central walkways, to which we have access in this study. In total, € 1.1 billion is spent until 2020 to enlarge the station, preparing it for future growth.

At the example of that case study, we investigate whether the proposed framework can provide an accurate understanding of the usage and level-of-service of pedestrian facilities. We thereby concentrate on walking areas, as for platforms already reliable dimensioning guidelines exist (Buchmüller and Weidmann, 2008). The analysis concentrates on the understanding of the status quo for two reasons. First, a validation can be made based on real data. Second, the predictions of demand and LOS we have made in collaboration with SBB are confidential.

The article is structured as follows. Section 2 provides an overview of the literature on ‘pedestrians in train stations’. Section 3 discusses types of data sources that provide information about rail access facilities and their use. Section 4 introduces the case study of Lausanne railway station. In Section 5, the estimation of pedestrian origin-destination demand is discussed, which allows to quantify the *usage* of pedestrian infrastructures. Section 6 considers the estimation of traffic conditions, allowing for the assessment of the *level-of-service* (LOS). Section 7 provides practical guidance for the use of these modeling tools in the dimensioning of rail access facilities, covering all stages of the planning process from the definition of the traffic concept of a train station to the verification of the dimensioning. Section 8 contains concluding remarks.

2 Literature Review

Questions related to the usage of rail access facilities increasingly attract the attention of academic research.

In an early study, Daly et al. (1991) investigate the relationship between speed and flow and between flow and travel time in various pedestrian facilities of London’s underground system. Lam and Cheung (2000) examine

several metro stations in Hong Kong. Differentiating by trip purpose, flow capacities are evaluated and flow-travel time functions are calibrated. Compared to the results from London, users of Hong Kong's mass transit system are found to be better at dealing with high levels of congestion, which is attributed to the 'smaller physique of Asians and their higher tolerance to invasion of space' (Lee and Lam, 2003).

Lam et al. (1999) investigate the train dwelling time and the distribution of pedestrians on platforms in two stations of Hong Kong's Light Rail Transit system. A behavioral analysis reveals that people are less willing to board a train if it is congested, and if the journey to be made is longer. Also focusing on train platforms, Zhang et al. (2008) describe the process of alighting and boarding in metro stations in Beijing. Pettersson (2011) investigates the behavior of pedestrians on railway platforms from an architect's perspective. At the example of a Swedish and a Japanese case study, the effect of signposts, availability of seats and entrances on the distribution of pedestrians along the platform is investigated.

Recently, Ganansia et al. (2014) have studied the use of standard CCTV networks for measuring pedestrian flows in train stations. Several case studies, including a TGV station and two subway stations in France and Italy, are discussed. It is found that data obtained through such a system is in principle useful for a continuous monitoring of the spatio-temporal evolution of pedestrian flows, but also that an a posteriori 'correction' is necessary whenever dense crowds need to be accurately measured. Using such camera-based data, Molyneaux et al. (2014) describe the flows on platform access ways caused by alighting train passengers. Similarly, van den Heuvel and Hoogenraad (2014) use automatic fare collection (AFC) data to investigate passenger arrival distributions.

Several studies have been dedicated to the understanding of route choice behavior (Seneviratne and Morrall, 1985; Borgers and Timmermans, 1986). For the case of a metro station in Hong Kong, Cheung and Lam (1998) investigate the route choice between escalators and stairways leading to a train platform. A relationship between flow and travel time is first established. This characteristic relationship is then used in a choice model to predict the percentage of escalator-users for ascending and descending directions as a function of prevailing traffic conditions. By 'shadowing' passengers, Daamen et al. (2005) collect route choice data in two Dutch train stations. Likewise, a route choice model is estimated,

allowing to predict the influence of level changes in walking routes on passenger route choice behavior. Further similar studies are provided by Srikukenthiran et al. (2014), Stubenschrott et al. (2014) and Ton (2014), who consider railway stations in Canada, Austria and the Netherlands, respectively.

Lee et al. (2001) provide one of the first model-based studies of pedestrian flows in train stations. For a major station in Hong Kong's metro system, origin-destination demand and travel times are collected using human observers. From this data, flow-travel time relationships are derived, which are used in a network-based pedestrian flow model. Along the same lines, Daamen (2004) develops a multitude of models for describing the processes of queueing, boarding, alighting, waiting, walking as well as route and activity choice.

Kaakai et al. (2007) develop a related model using a Petri net. They consider both discrete processes such as the arrival and departure of trains, as well as continuous processes such as the 'fill-up' of railway platforms by pedestrians awaiting a train, or pedestrian flows in walking facilities. The model is applied to a French case study involving a train station with a single platform. At the microscopic level, Xu et al. (2014) develop a model describing pedestrian behavior in a Chinese metro station. The framework is entirely based on a queueing network, i.e., all processes including entering the train station, passing ticket gates, walking and boarding are represented by queues. The framework is applied to estimate the maximum service rate of a metro station, as well as to determine the optimal inflow rate at the entrance at which this capacity is attained.

There are several more studies of pedestrian flows in train stations that concentrate on specific applications. Most of them pursue an agent-based approach and describe various practical challenges such as the placement of access gates in Lisbon (Hoogendoorn and Daamen, 2004), the re-design of access ways in Bern (Rindsfuser and Klügl, 2007), the evacuation of a metro station in Beijing (Jiang et al., 2009), the modeling of waiting areas in German train stations (Davidich et al., 2013), the design of a new station in South Africa (Hermant, 2012), or, based on a macroscopic 'pedestrian transfer chain', the assessment of an existing station in the Netherlands (Starmans et al., 2014).

These models typically use relatively simple approaches to estimate pedestrian demand, such as theoretical demand scenar-

ios (Hoogendoorn and Daamen, 2004; Rindsfuser and Klügl, 2007; Davidich et al., 2013), or rules of thumb (Kaakai et al., 2007; van den Heuvel and Hoogenraad, 2014). There are a number of studies focusing on origin-destination (OD) demand estimation, but typically at the level of transit networks (Nguyen et al., 1988; Wong and Tong, 1998; Lam et al., 2003b; Montero et al., 2015). These are useful to predict the evolution of in- and outflows at stations or the number of passengers in vehicles, but do not provide any information about OD demand *within* a train station. We have recently proposed an alternative approach (Hänseler et al., 2015b), which is applied to the example of Lausanne railway station in Section 5.

As for other transportation modes, an assessment scheme for pedestrian facilities exists that allows to quantify the quality and comfort of pedestrian traffic. The corresponding literature on the assessment of level-of-service (LOS) is dominated by the seminal contribution by Fruin (1971), who proposes a density- and flow-based classification considering six service levels. Density-based LOS indicators are useful both for walking and waiting areas, for which different thresholds apply. Flow-based indicators are used for walkways, escalators or stairways, and consider the specific flow, i.e., the flow per meter of width. Several other assessment schemes have been proposed, typically focusing on the integration of additional factors such as safety, aesthetics and comfort, or taking the opinion of pedestrians into account (Polus et al., 1983; Mōri and Tsukaguchi, 1987; Khisty, 1994). Due to their more difficult use, Fruin’s classical LOS classification schemes have mostly prevailed in practice, even though minor modifications have been made that consider national differences (Highway Capacity Manual, 2000; Brilon, 2001). The US-American standard, which is also applied in Switzerland, is further discussed in Section 6.

3 Data sources

Monitoring pedestrian traffic is difficult. First, the placement of sensors is challenging, as pedestrians can explore space freely, and are not confined to lanes. Second, the detection of pedestrians is an intricate task, as they can almost instantaneously stop or accelerate, and often travel in groups. Third, pedestrian traffic is highly variable, and sensors are required to

capture a large range of traffic levels (U.S. Department of Transportation, 2013). Data availability is thus often limited in terms of its spatial or temporal coverage, or in terms of quality.

In the following, we provide a classification of data sources in five data types (see also Hänseler et al., 2015b), and discuss the importance of each data type for demand estimation and LOS assessment. For a discussion of sensing technologies as well as practical guidance, we refer to the literature (Turner et al., 2007; Bauer et al., 2009; U.S. Department of Transportation, 2013).

OD flow data: Origin-destination (OD) trip tables represent the number of people traveling between each pair of origin and destination during predefined time intervals. The definition of OD areas depends on the layout of a train station, and may include platforms or platform sectors, shops, as well as entrance/exit areas. By convention, pedestrians are counted when they leave their origin.

OD flow data are obtained from pedestrian tracking systems, travel surveys, electronic tickets, or passive ICT sensors such as Bluetooth and WiFi scanners (Versichele et al., 2012; Alahi et al., 2013b; Kim et al., 2015). Due to their expensive collection, OD flow data are often not available for the entire network of interest (Bauer et al., 2009). Moreover, sampling is typically an issue, as in practice only a subset of pedestrians may be successfully detected.

OD flow data are of particular importance for OD demand estimation, where they help to reduce the underdetermination that results if only indirect indicators of demand, such as link flows, are available (Cascetta et al., 1993). For LOS assessment, OD flows can also be useful if a traffic assignment model is available that allows to estimate LOS indicators.

Link flow data: Pedestrian infrastructures are often represented as a flow network, consisting of nodes and links. Links include in particular walkways or walkway sections, stairways, or escalators. By convention, ‘link flow’ refers to the inflow to a link, i.e., the flow that is measured at the origin of a link. Link flow data may be obtained from turnstiles, camera-based systems, infrared sensors or other detectors, including manual counting (Lee et al., 2001; Ton, 2014; Kim et al., 2015). Compared to OD flow data,

the sensor technology for obtaining link flows are relatively mature, and the counting precision is high (U.S. Department of Transportation, 2013).

For OD demand estimation, link flow data represent the most common type of input data (Cascetta and Improta, 2002). As mentioned in the introduction, the efficient placement of sensors within a network is difficult, and widely discussed in the literature (Gentili and Mirchandani, 2012; Viti et al., 2014).

There are two ways of using link flow data for LOS assessment. Directly, by computing the specific flow along a link, and by comparing that to facility-specific thresholds (Fruin, 1971). For simple geometries, such as straight corridors with a constant width, this may be appropriate. A second way of using link flow data for LOS assessment is by using a traffic assignment model, applying it to the link of interest.

Traffic condition data: Traffic condition data include measurements of density, walking speed, or travel times. Such observations are typically obtained from a pedestrian tracking system, or ICT sensors (Alahi et al., 2013b; Montero et al., 2015).

For the estimation of pedestrian OD demand, traffic condition data can be used as exogenous variables in the estimation process. For instance, Montero et al. (2015) use observed travel times to approximate the travel time distribution within a demand model. Alternatively, they can be used to indirectly validate the OD demand estimates, if the latter are combined with a traffic assignment model (see e.g. Djukic et al., 2015).

Traffic condition data are probably the most relevant source of information for LOS assessment. The most widely used LOS indicators are directly based on density and specific flow (Fruin, 1971).

Train timetable and ridership data: The train timetable has a significant impact on the usage of pedestrian facilities, both in terms of accumulation and in terms of flows. Fig. 2 provides a schematic representation of the most relevant types of flows that are influenced by the train timetable, namely boarding/alighting flows at train doors, as well as exit and access flows on platform access ways. A direct relationship between the train timetable and platform exit flows is established in Section 5.

Highly-interconnected timetables, or railway networks during peak periods, are prone to delays (Cule et al., 2011). If available, the actual instead

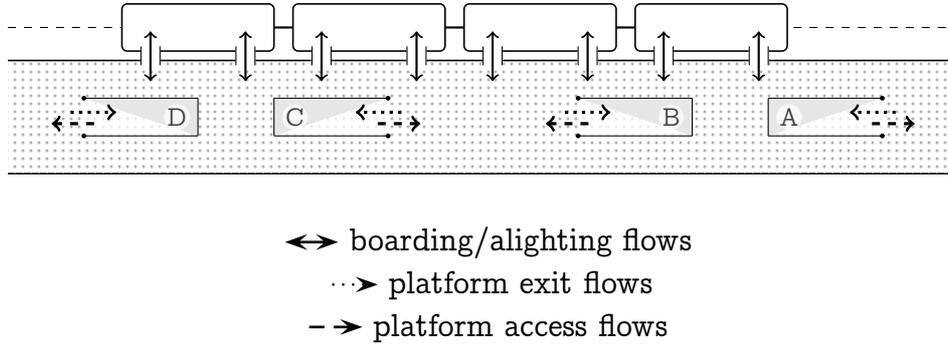


Figure 2: Train-induced flows on platforms and platform access ways.

of the scheduled train timetable should be used. Alternatively, train delays may be explicitly modeled (Higgins and Kozan, 1998; Goverde, 2007).

The number of boarding and alighting passengers per train may be obtained from door sensors, ticket sales, travel surveys, or approximated from the train capacity (Zhang et al., 2008; Kim et al., 2015; Fernández et al., 2015).

For OD demand estimation, the timetable and ridership information can be used to improve the accuracy of the estimate, or to provide a priori estimates when no other data is available (Hänseler et al., 2015b). In terms of LOS assessment, the number of boarding and alighting passengers is particularly useful for the dimensioning of platforms, which in practice is typically done using hydrodynamic models (Buchmüller and Weidmann, 2008).

Other data: Other information sources, such as sales or survey data, are sometimes available (Seneviratne and Morrall, 1985; Lee et al., 2001). These are typically useful for demand estimation, where they help narrowing the solution space. Video footage or a photographic documentation may be helpful for a qualitative level-of-service assessment (Helbing et al., 2002). Finally, practical knowledge by the operator of a train station regarding pedestrian dynamics, critical areas and the use of infrastructure may be a useful source of information as well.

4 Case Study

Lausanne railway station is the largest node in the railway network of Western Switzerland, serving 650 arriving and departing trains on weekdays (Amacker, 2012). Fig. 3 shows a schematic map of the station, encompassing nine railway tracks for passenger traffic. At its heart are two pedestrian underpasses (PUs), referred to as PU West and PU East. Solid lines represent a network representation of pedestrian facilities. Dashed lines are network links that cannot be represented in the 2D scheme. Pedestrian count sensors are represented by diamonds. The shaded areas in the two pedestrian underpasses are covered by a pedestrian tracking system.

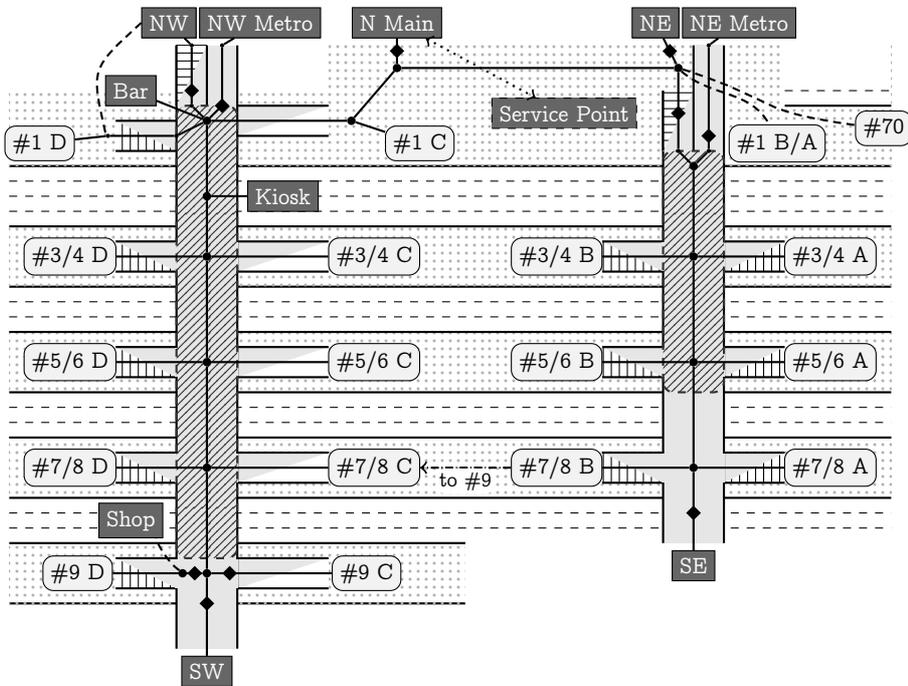


Figure 3: Lausanne railway station.

Using the classification presented in the previous section, the following data sources are available:

OD flow data: Subroute flows are available for the two pedestrian underpasses, in which a tracking system consisting of 60 sensors is installed (Alahi et al., 2013b).

Link flow data: Ten links of the pedestrian walking network, marked by diamonds in Fig. 3, are equipped with sensors that provide directed link counts with a resolution of one minute.

Traffic condition data: Pedestrian trajectories obtained from the aforementioned tracking system allow to compute the prevailing speed, density and accumulation in pedestrian underpasses. Accumulation is defined as the number of pedestrians present in an area at a given point in time.

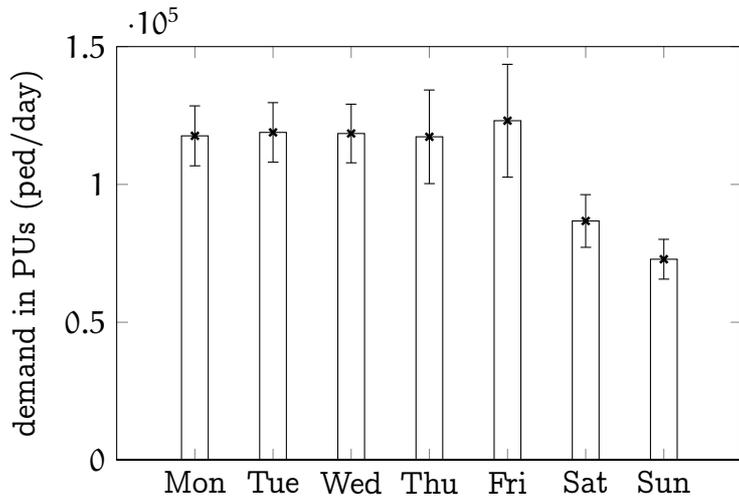
Train timetable and ridership data: The actual arrival and departure time and the assigned track are known for each train. An average estimate of boarding and alighting volumes is available from ticket sales data, within-train surveys, and infrared-based counts at train doors (Anken et al., 2012). These estimates date back to the year 2010 and are increased by 15% to reach the estimated level of 2013 (Gendre and Zulauf, 2010).

Other data: For the sales points located in PU West (see Fig. 3), an estimate of the number of customer visits is available.

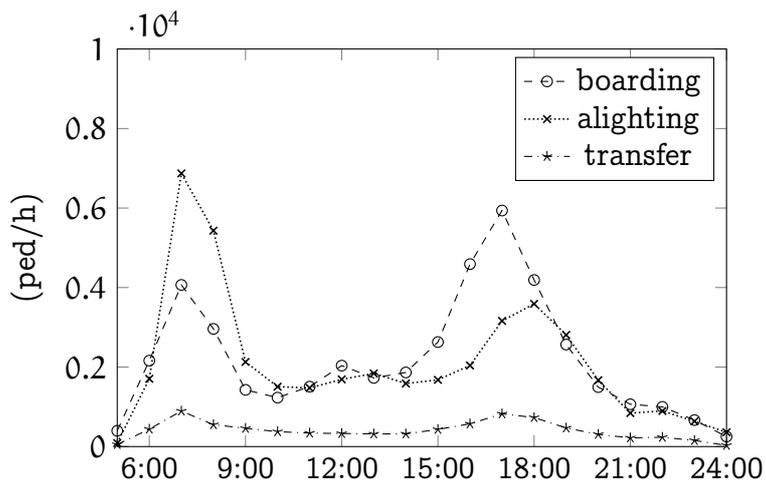
The usage of pedestrian facilities in Lausanne railway station is subject to recurring temporal patterns that are due to differences between weekdays/weekend, the day/night-rhythm, and a cyclic train timetable.

Fig. 4a shows the level of demand in the PUs over a typical working week, as measured by the pedestrian tracking system for the period between February 25 and May 19, 2013 (April 1 and 2 are excluded due to a sensor malfunctioning). Standard deviations are around $\pm 15,000$ pedestrians for a typical working day.

The total number of pedestrian visits in the two pedestrian underpasses of Lausanne railway station (PU West and East) is slightly below 120,000 ped/day on weekdays. This is in agreement with numbers reported by SBB, according to which there are in total about 140,000 station users per weekday, of which 98,000 are train users (Amacker, 2012). On Fridays, the station is busier than during the week due to weekly commuters returning to their principal place of residence, as well as due to weekend travelers. These additional passengers are spread around the evening peak period. The pedestrian demand on Saturdays and Sundays on the other



(a) Demand in pedestrian underpasses over a week



(b) Train passenger volume over a day

Figure 4: Observed demand in Lausanne railway station (year 2013).

hand is significantly lower. The shown pattern is similar to other major train stations in Switzerland, including in particular Basel, Bern and Zürich, which are serving up to four times as many passengers.

Fig. 4b shows the evolution of train passengers during the course of a weekday. The shown data is obtained from semi-automatic travel surveys conducted in the year 2010, increased by 15% to approximate the demand in 2013 (Anken et al., 2012). It is distinguished between outgoing passengers (boarding), incoming passengers (alighting), and transfers, i.e., passengers

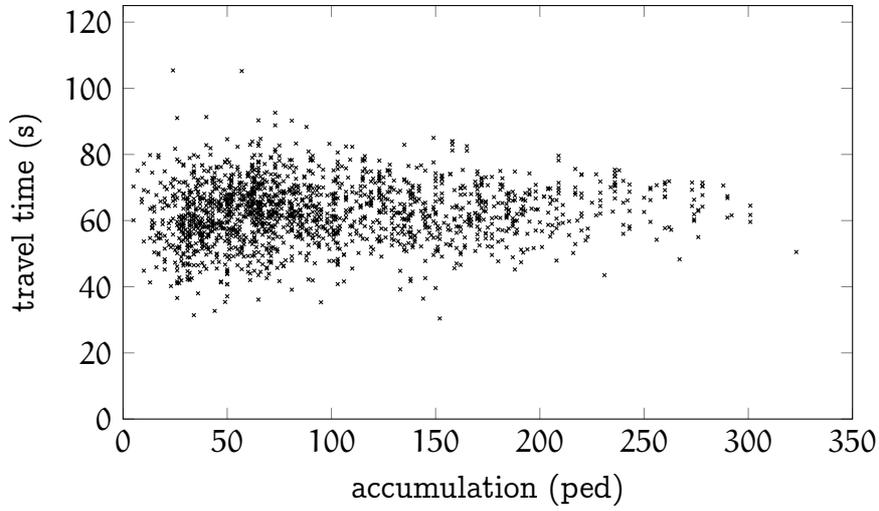
that change train in Lausanne.

Between 7:00 and 8:00, the alighting volume (6,871 ped) is higher than the boarding volume (4,066 ped), whereas in the evening rush hour between 17:00 and 18:00, the number of boardings is higher (3,161 vs. 5,937 ped). According to these results, people come to Lausanne for work and leave the city again in the evening. The morning peak hour is shorter and busier than the evening peak hour, while the percentage of transfer passengers is just below 10% and nearly constant during the day. The bi-modal distribution of train passengers with a distinct peak in the morning and evening is typical for most train stations, with the exception of those that are primarily used for timed events such as concerts, or for touristic purposes.

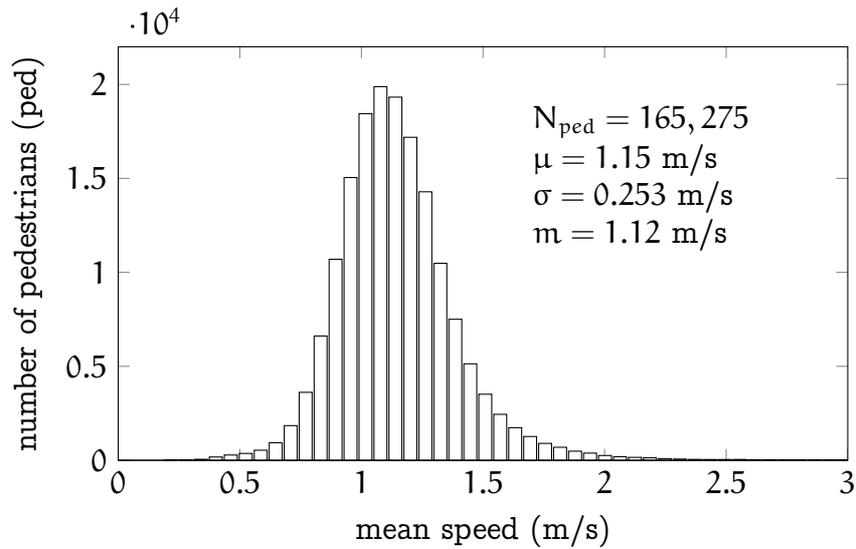
A further analysis of the morning peak hour shows that the absolute peak over a weekday is reached between 07:35 and 7:50 (AM), when several long distance trains arrive and depart in close succession (Gendre and Zulauf, 2010). At this time of the day, more than 500 incoming users alight during a peak minute, whereas a few minutes later it can be less than a hundred per minute (Alahi et al., 2013a).

In the ensuing analysis, we consider the time period between 07:30 and 08:00 with a temporal aggregation of one minute. Data for a set of 10 ‘reference weekdays’ is available, namely for January 22 and 23, February 6, 27 and 28, March 5, as well as April 9, 10, 18 and 30, 2013. These dates represent a set of typical weekdays (Tue, Wed, Thu) without major disruptions in the railway system, for which all of the aforementioned data sources are available.

Fig. 5a shows a scatter plot of accumulation vs. travel time along the main route in PU West, #1C \rightarrow SW. The data points represent 1,745 pedestrians. The accumulation is measured when a person enters PU West. The distribution of travel times is relatively wide, particularly at low values of accumulation. The mean travel times do not depend significantly on the accumulation, as would be the case if the facility were congested. This is an important finding for the demand estimation discussed in the next section, where data sources from different locations are combined. In the case of Lausanne railway station, the ‘temporal distance’ between sensors remains approximately constant across time, irrespective of the demand. It is unclear to what extent this finding can be generalized to other train stations. Presumably, it holds for most other train stations with a low to moderate level of congestion. For highly congested train stations, such



(a) Correlation between accumulation and travel time on route #1C → SW



(b) Walking speed during morning peak hour in pedestrian underpasses

Figure 5: Travel times and walking speeds.

as they are found in large cities, travel times are likely to depend on the prevailing densities.

Fig. 5b shows the walking speed distribution observed in the monitored areas of Lausanne railway station. For a total of 165,275 pedestrians, the walking speed is computed from the ratio of traveled distance and

travel time. A mean velocity of 1.15 m/s is observed. The median lies at 1.12 m/s. These values are in good agreement with the literature. For instance, Weidmann (1992) reports for the mean speed a range between 0.99 m/s for tourists, up to 1.45 m/s for business people. The spreading of the walking speed distribution is largely caused by differences in trip purpose, as well as by population heterogeneity in terms of age and gender. Other factors, such as time pressure, whether luggage is carried, or general health may also play a role.

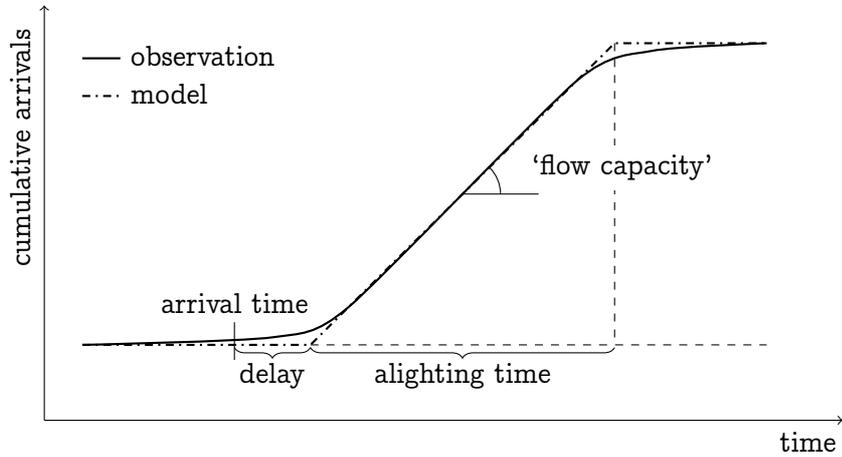
5 Estimation of origin-destination demand

An explorative data analysis allows to identify the busiest time period and to accordingly reduce the estimation problem from a full day to a peak period. To obtain a comprehensive understanding of pedestrian demand during that period, the different data sources have to be ‘combined’ in an estimation framework. This is the focus of this section. For a mathematical description of the used OD demand estimation framework, the reader is referred to Hänseler et al. (2015b).

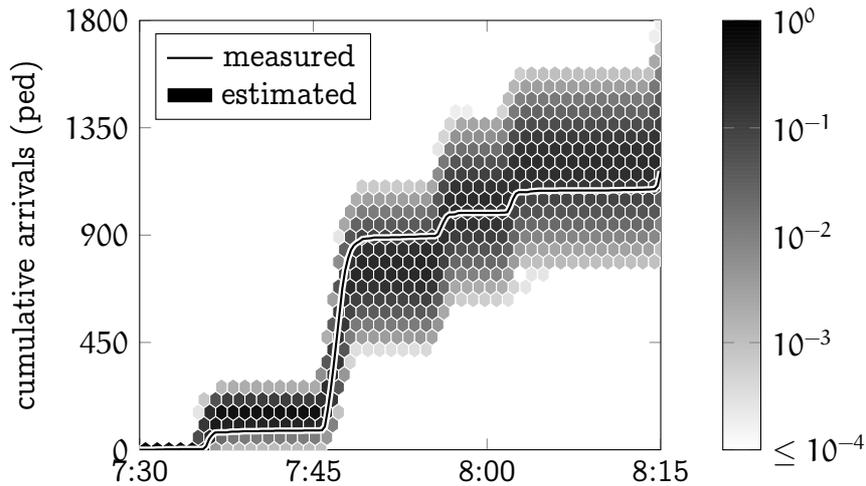
The problem of estimating OD demand consists in finding an estimate that, when applied to the pedestrian network of a train station, is ‘most consistent’ with the corresponding train timetable, historical surveys, and all other data sources that are available (Cascetta and Improta, 2002). In the case of Lausanne railway station, all the aforementioned data sources are used with the exception of pedestrian trajectory data, which is considered for validation only.

In line with the introduction, we focus on the usage of walking facilities, and we take the train timetable explicitly into account. For that purpose, we concentrate on platform exit flows that are caused by alighting passengers of arriving trains. These are known to cause demand ‘micro-peaks’ that are critical for the dimensioning of rail access facilities (Hermant, 2012). Fig. 6a illustrates the typical pattern of platform exit flows (dash-dotted line), as well as a corresponding piece-wise linear model that is derived from it (solid).

After the arrival of a train, a certain time elapses until the first pedestrians reach the platform exit ways. This may be due to the necessary walking to reach the exit ways, or a delay in the opening of doors after



(a) Sample observation and piece-wise linear model



(b) Prediction and measurement for April 10, 2013 at platform #5/6

Figure 6: Flow of alighting passengers on platform exit ways.

the train has stopped. Subsequently, a constant flow is established, whose magnitude is limited by the capacity of the exit ways. This assumption is based on empirical observations, showing that the exit ways typically represent the bottleneck in that situation (Benmoussa et al., 2011). Depending on the number of available exit ways, which often is determined by the position and length of a train, the magnitude of the flow may be different. Once all alighting passengers have left the access ways, the flow

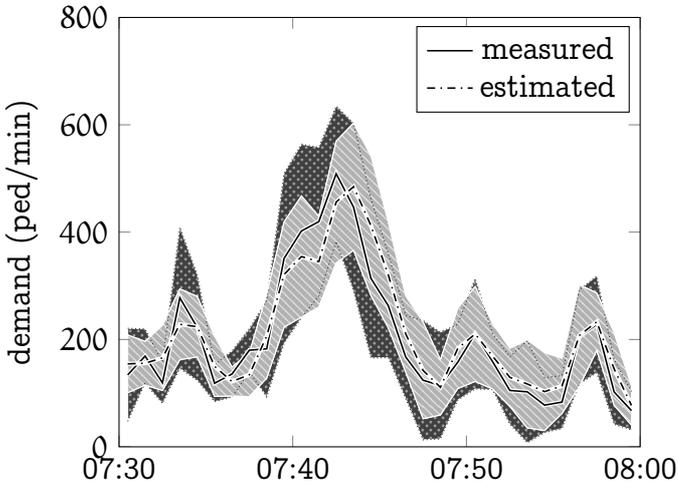
reaches again zero.

Due to various random effects, such as natural fluctuations in the ridership of a train, or its position along a platform, the parameters of the piecewise linear model are stochastic variables. The prediction of the model is then also stochastic, and may be represented by a probability band. In Fig. 6b, such a prediction band is shown for the exit flows from platform #5/6, together with the actual measurement. The probability band represents the expected cumulative arrivals using a logarithmic probability density, and has been obtained from 7,500 Monte Carlo samplings. A good agreement between the prediction and the observation is found. The width of the band indicates that the variation in alighting volumes across days is relatively high.

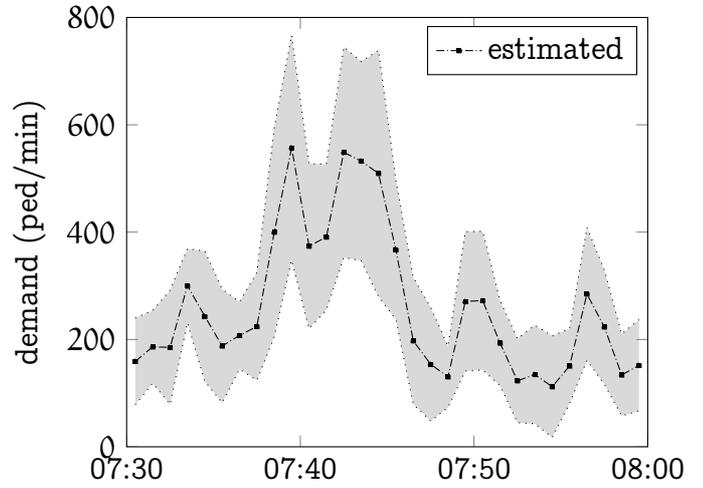
The model can be applied to any platform if an appropriate specification of its parameters is available. In the case of Lausanne railway station, such a parametrization is available, and train-induced exit flows can be predicted for all platforms (Molyneaux et al., 2014). These flows are used in the OD demand estimation framework, together with the available pedestrian counts and sales data. To associate the different information sources over space and time, a Normal walking speed distribution is assumed. The specifications for even walkways, inclined areas and stairways proposed by Weidmann (1992) are used.

The OD demand is jointly estimated for the 10-day reference set. Fig. 7a provides a comparison of the estimated demand in the two pedestrian underpasses, and the corresponding observation obtained from the tracking system. The mean and standard deviation band are shown for both sources. Despite the strong and rapid fluctuations, the measured mean lies within the prediction band throughout the considered time horizon. An analysis shows that the differences between the prediction and measurement for individual days is smaller than the day-to-day variability observed in measurements of tracking data, which is used for validation only. Thus, at least for dimensioning purposes, the estimate of total demand can be considered accurate. Similar findings hold for the estimation of accumulation or OD flows (Hänseler et al., 2015b). In all cases, the integration of the train timetable is essential to reach such accuracy.

In Fig. 7b, the estimated evolution of the total demand is provided. Both the within-day and the day-to-day variation (as indicated by the width of the prediction band) are significant. The average cumulative demand



(a) Demand in PUs (MAE = 30.03, RMSE = 37.56)



(b) Estimate of total demand in station

Figure 7: Estimated demand during morning peak hour.

over the studied 30-min period amounts to 7,906 ped, representing about 8% of the daily station throughput (Amacker, 2012). The highest average demand is found between 7:39 and 7:40, where the overall demand rate amounts to 557.3 ped/min. A quarter of an hour later, between 7:54 and 7:55, the mean demand reaches a minimum of 112.0 ped/min. Within only a couple of minutes, the average demand thus varies by almost a factor of 5. Such a periodical concentration is characteristic for the Swiss railway network that aims at bundling train arrivals and departures in order to minimize waiting time for transfer passengers (SBB-Infrastruktur, 2013).

To consider the spatial distribution of demand, the latter may be aggregated over time. Fig. 8 shows a ‘Circos’ diagram of the average pedestrian OD demand (Krzywinski et al., 2009). Origin/destination areas are grouped into ten centroids, representing the railway platforms #1, #3/4, #5/6, #7/8, #9 and #70, the entrances North and South, the passageway to the metro, and a collection of shops. Light gray strips represent pedestrian flows emanating from railway platforms, medium gray those originating at the entrance ways North and South as well as at the interface to the metro station, and dark gray strips pedestrian demand emanating from one of the sales points.

Circos diagrams have originally been developed for studying genomes (Krzywinski et al., 2009), but turn out to be a powerful instrument for con-

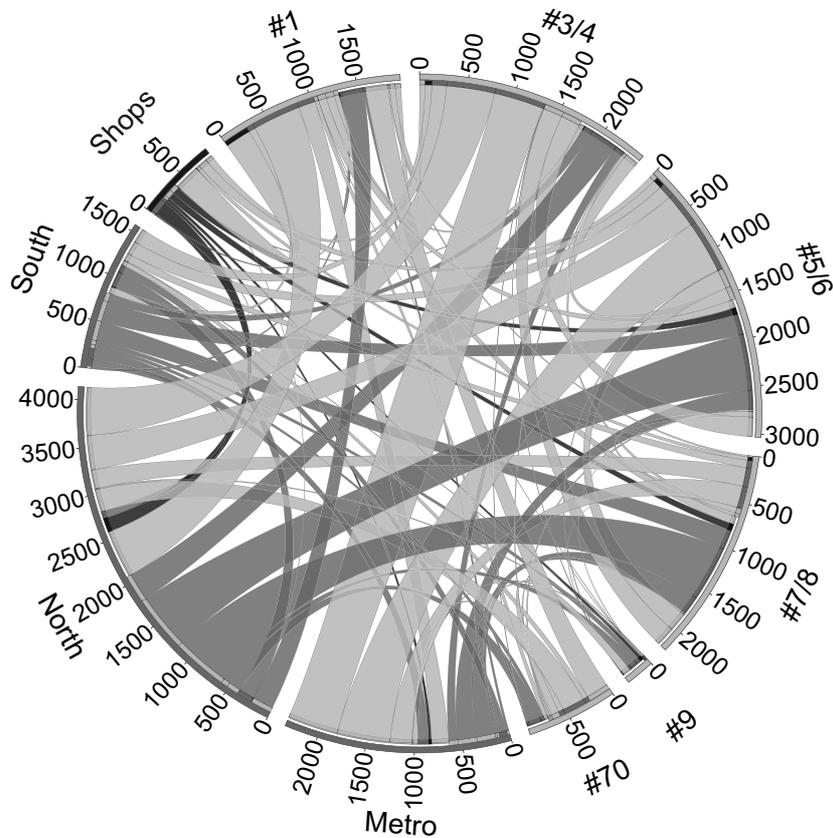


Figure 8: Pedestrian OD demand between 07:30 and 08:00. The origin of strips are color-coded as train platforms (light gray), city/metro/bus (medium gray) and shops (dark gray).

veying the spatial structure of pedestrian OD demand to practitioners and authorities. They provide quantitative information of flow between any two centroids, which usually is difficult to represent in a single diagram. Moreover, the share of different user classes can be immediately perceived based on the different shadings. During the considered time period, 44.1% of all station visitors represent inbound passengers, 31.2% represent outbound passengers, 16.4% are transfer passengers, and the remaining pedestrians represent local users. These figures are different for each train station, and change between the morning, evening and off-peak periods.

A further way of visualizing demand is by means of network flows. Fig. 9 shows a map of the estimated minute-by-minute link flows for the time period between 7:40 and 7:48 on April 30, 2013. Here, the demand

estimate of a specific day is chosen, as it allows to visualize the demand peaks caused by individual train arrivals and departures. The shading of links represents the cumulative link flow over a minute in both directions. The diameter of nodes represents the minute-by-minute origin flow.

Between 7:40 and 7:41, the arrival of IR 1712 from Sion at 7:38:57 is discernible by the origin flow it creates on platform #5/6. In the time period considered, this train is among those with the highest alighting volumes. During 7:41 and 7:42, the arrival of IR 1606 from Neuchâtel on track #4 can be seen by the trace it leaves in the pedestrian flow map. Within less than a minute, IR 1710, IC 706 and IR 1407 arrive on platform #7 at 7:42:24, platform #5 at 7:42:59, and on platform #3 at 7:43:18, respectively. Especially the former two represent major lines (from Brig and Zürich), causing large pedestrian movements. Their impact is visible in Fig. 9d and 9e. After the last arrival of a train, IR 2517 from Geneva arriving on platform #1 at 7:44:37, pedestrian flows decay, as can be seen from Fig. 9g and 9h.

Flow maps are useful to get an intuitive understanding of the spatio-temporal distribution of demand. They allow to see in a visually compelling way through which origin/destination area pedestrians are entering the station, and which links are most frequented at a given point in time.

6 Level-of-service assessment

Origin-destination demand alone reveals little about expected traffic conditions. To assess the level-of-service, the interaction between infrastructural supply and demand needs to be taken into account.

For that purpose, a dynamic traffic assignment model is necessary. In the following, such a model is described and applied to investigate density levels in PU West, the busiest area in Lausanne railway station (see Fig. 9), and the narrower of the two pedestrian underpasses. A macroscopic modeling approach is pursued for three reasons. First, it allows to easily infer LOS indicators like speed and density, which typically is the state variable in aggregate models. Second, such models have a low number of parameters, which simplifies the calibration on real data (Hoogendoorn and Daamen, 2007). Third, large networks can be assessed in little time, which is particularly useful if several scenarios need to be studied, or for time-critical

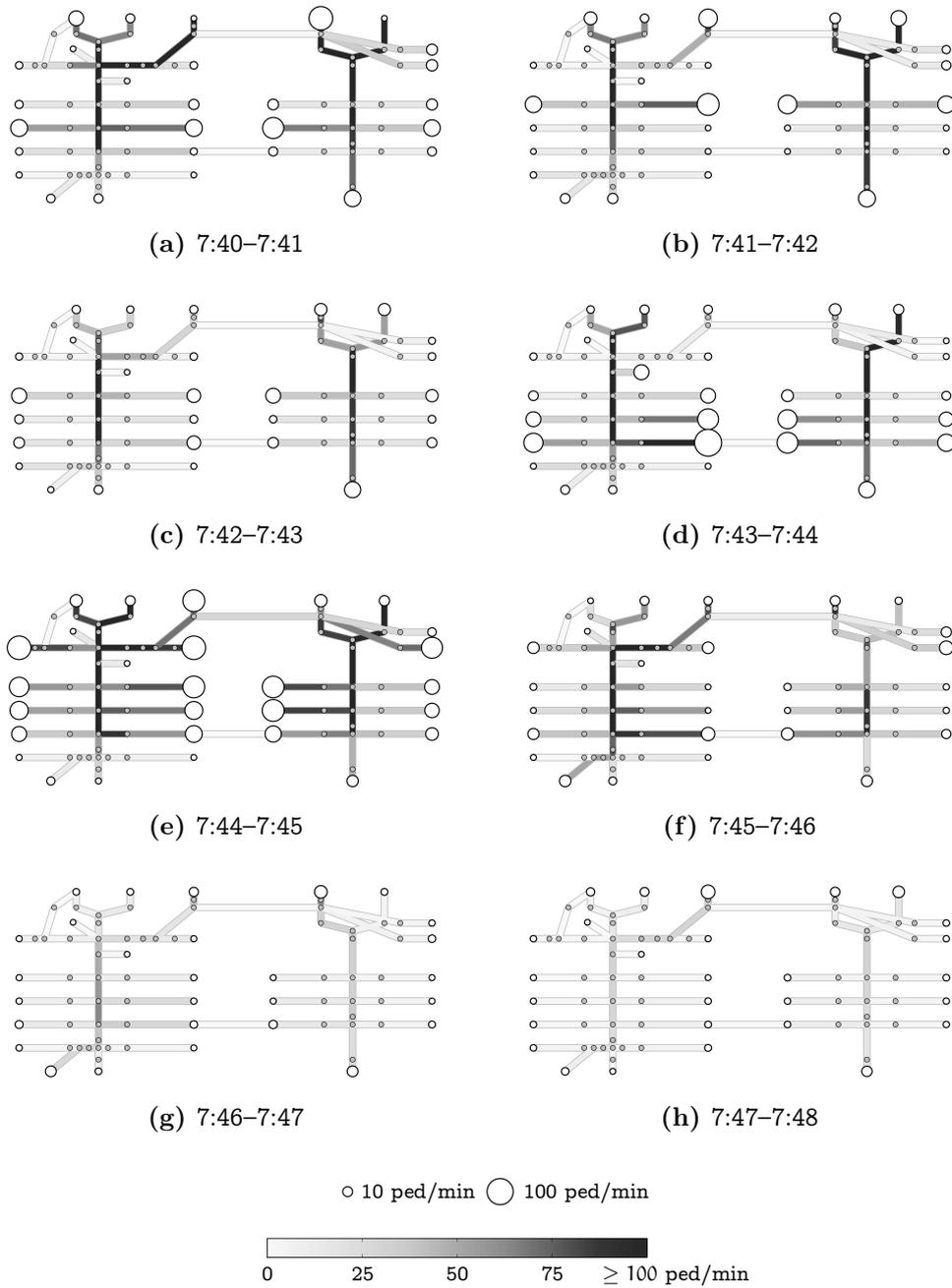


Figure 9: Pedestrian flow map between 07:40 and 07:48 on April 30, 2013.

A pedestrian fundamental diagram governs the propagation of pedestrians across cells. Fig. 10b shows the used density-speed relationship (solid curve), as well as the corresponding density-flow relationship that results in case of uni-directional motion (dashed curve). The functional form of these relationships is as proposed by Weidmann (1992), and the parametrization is obtained from a calibration on trajectory data collected in Lausanne railway station in April 2013.

As can be seen from the density-flow relationship, the domain is split into a free-flow and a congested regime, with the transition at a density of 1.86 ped/m^2 . An increase in density in the free-flow regime leads to an increase in flow, whereas in the congested regime the opposite occurs. Such a behavior is typical for transportation networks, and applies both to vehicles and pedestrians (see e.g. Geroliminis and Daganzo, 2008).

For each cell at given time intervals, the density is calculated, and based on the fundamental diagram, the corresponding flow is computed. It is thereby assumed that the walking speed within a cell is the same in all directions, i.e., isotropy of walking speed is assumed. This is a fair assumption for mildly congested flows or those that are inherently uni-directional. In case of multi-directional flow at high densities, an anisotropic network loading model should be used (Lam et al., 2003a; Hänseler et al., 2015a).

The density in cells can be directly used to assess the perceived comfort and performance of a facility. The Highway Capacity Manual (HCM Highway Capacity Manual, 2000) distinguishes six levels of service, ranging from LOS A (below 0.18 ped/m^2 , most favorable) to LOS F (above 1.33 ped/m^2 , least favorable).

Fig. 11 shows the resulting level-of-service maps for January 22, 2013. For each time period of one minute, the model estimates and the corresponding measurement from pedestrian tracking data are shown.

Visually, the proposed traffic assignment model is able to reproduce the trend of the actual measurements. In the first time interval, the density maps show a high level of service, which is then reduced during the following minutes, before it improves again in the last interval. There are certain differences, for instance regarding the concentration of pedestrians along the center line of the corridor, which is less distinct in the model prediction than in the measurement. An analysis of several days shows that these differences between model prediction and measurement are relatively small compared to the day-to-day variation. For the purposes considered

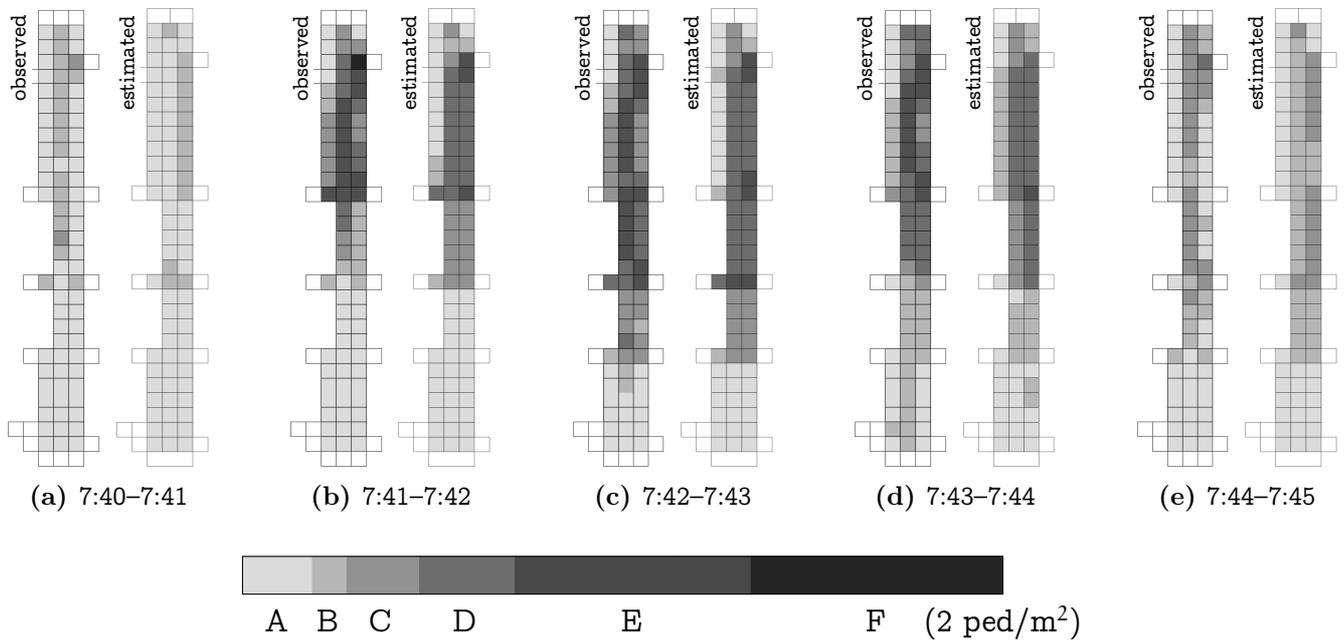


Figure 11: Level-of-service in PU West on January 22, 2013.

in this work, i.e., for an assessment of the level-of-service that is sufficient for dimensioning, the estimates are again considered accurate. Moreover, a comparison to the social force model, which is the state-of-the-art in microscopic pedestrian flow modeling, shows that the performance of the proposed macroscopic model is equivalent or superior (Helbing and Molnár, 1995; Hänseler et al., 2014a).

The highest pedestrian densities are observed between 7:41 and 7:43, when various trains arrive. The level-of-service lies in the range between A and E, i.e., densities are generally below 1.33 ped/m^2 . As is discussed in Section 7, according to Swiss standards LOS E should only be tolerated in bottlenecks, and not be present on wide walkways as in the case of PU West. The capacity of PU West is thus insufficient not only for the future, but already for the current demand.

To assess the required transfer times between connecting trains, walking times are of interest. Fig. 12 shows the walking time distribution in PU West as estimated by the traffic assignment model, and as observed in the trajectory data. A good qualitative agreement is found. Further analysis shows that this also holds for route-specific predictions of walking times, and in case of congestion (different case study, see Hänseler et al., 2014a).

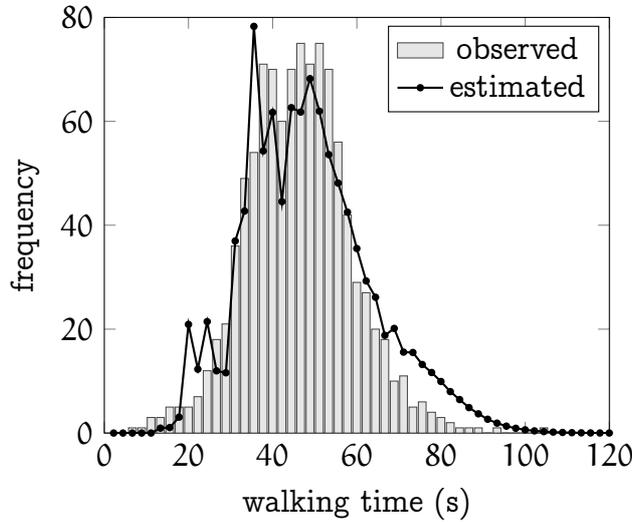


Figure 12: Walking time distribution for PU West ($\Delta t = 2.22$ s).

The cell-based pedestrian traffic assignment model can thus be used to accurately predict the level-of-service in walking facilities of a train station. As before, no tracking data is necessary, of which only an independent data set has been used for calibration. Even if the model is calibrated solely based on values from the literature, its predictive quality is still good. This is due to the fact that the obtained parametrization is very similar to those of other researchers (see e.g. Weidmann, 1992).

7 Planning guidelines

In practice, the estimation of the current demand and level-of-service may be of interest, but typically does not represent the primary objective. More relevant is the consideration of future scenarios, which allow the assessment and optimization of construction alternatives, and thus the dimensioning of infrastructure facilities.

The configuration and dimensioning of pedestrian facilities in train stations is traditionally based on artificial scenarios, such as ‘passengers on platform awaiting boarding’, ‘flow of disembarking passengers on platform exit ways’, or ‘transfer flows on pedestrian walkways’ (Hoogendoorn and Daamen, 2004; Buchmüller and Weidmann, 2008; Zhang et al., 2008). In structural engineering, these scenarios are referred

to as ‘load cases’. An infrastructure is checked for serviceability against all the load cases it is likely to experience during its lifetime. In the context of rail access facilities, the load cases typically consider the arrival or departure of one or a few reference trains, and the resulting pedestrian OD demand is estimated using rules of thumb. The dimensioning is then done manually and separately for each facility element, such as stairways, ramps, walkways or platforms.

By using a computational framework to estimate demand and level-of-service, the planning process of rail access facilities can be enhanced. First, due to the explicit integration of the full train timetable, the use of individual load cases becomes obsolete. Second, the various facility elements can be dimensioned jointly, which allows to investigate their mutual influence on each other.

Based on our experience from Lausanne railway station, we suggest in the following a six-step process that is useful for the planning and dimensioning of pedestrian facilities in train stations, be it existing ones, or new stations. The approach is based on guidelines by Buchmüller and Weidmann (2008). The differences consist in (i) the direct consideration of the usage of a train station based on the timetable instead of indirectly through load cases, (ii) the use of a computer-based OD demand estimation framework instead of manual estimation techniques, and (iii) the use of a pedestrian traffic assignment model that allows to simultaneously dimension multiple facility elements. For each of the six steps, a short illustration at the example of Lausanne railway station is provided. The process may be iterative.

I. Traffic concept of train station. In a first step, the planning horizon is to be determined, as well as the corresponding operational concept for the expected peak periods. This includes the train timetable or line frequency, as well as the capacity and type of rolling stock. In Swiss train stations, typically the morning peak period on working days is critical, and in rare cases the evening peak hour. In particular cases, such as in touristic areas or for stations close to stadiums, certain periods on weekends or after mass events may be decisive for the dimensioning.

In a second step, a preliminary prediction of pedestrian OD demand can be made. This typically requires an analysis of the status quo, which serves to calibrate the demand estimator (see Section 5). Unless sufficient infor-

mation is available, a data collection campaign may be required, involving for instance manual travel surveys, or the installation of flow sensors.

In the case of the on-going expansion of Lausanne railway station, the planning horizon is the year 2030, for which detailed information of the train timetable and rolling stock is available. The demand is 'expected to double for interregional trains, and to triple for regional trains' (Caillaud, 2011). To better understand the usage of the train station, a pedestrian tracking system has been installed. An exploratory data analysis, discussed in Section 4, shows that the critical period occurs indeed in the morning.

II. Functional requirements. The desired level-of-service needs to be specified, for instance for walkways, stairways, platforms or waiting areas. Typically, one of the standard LOS schemes is used, which rely on density or specific flow. In accordance with Swiss and US-norms (Weidmann, 1992; Highway Capacity Manual, 2000), it is typically required that LOS B or better be maintained for intervals of several minutes. During short intervals of up to a minute, LOS D is accepted. At bottlenecks, locally LOS E is still tolerated. Separate standards may apply under particular circumstances, such as after mass events. Maximal walking times can also be set, either based on a preliminary timetable that requires certain transfer times, or based on considerations related to comfort. The particular needs of handicapped train users need to be taken into account, in accordance with the local legislation.

The placement of service and sales points needs to be discussed. Access to such facilities may increase the comfort and well-being of train station users, but at the same time compromise pedestrian traffic. The effect of the latter should be taken explicitly into account in the dimensioning of pedestrian facilities, both as far as available space is used, or additional demand is induced. Generally, the more important a service, the higher its priority in the allocation of space should be, however without violating the pre-defined LOS standards.

In the case of Lausanne, maximum acceptable service levels are defined for platforms, ramps, stairways and horizontal walkways. The density-based LOS schemes specified in the Highway Capacity Manual (2000) are used, with the thresholds as mentioned above (generally LOS B or better, for short intervals LOS D, LOS E exceptionally at bottlenecks). For the placement of sales and service points, the status quo is preserved.

III. Topology of pedestrian facilities. The network of pedestrian facilities is to be developed. This process takes into account (i) the surroundings of the train station, and in particular factors such as the connection to the local transportation system, the local network of walkways, points of attraction in the vicinity of the station, and workplace locations; (ii) existing buildings that are to be preserved, such as historical station halls or facilities that do not require a structural extension; and (iii) the track topology, which is either the existing one, or imposed by the design of the future rail network.

Subsequently, the type of facility elements and the connection between them can be specified. For vertical level changes, a choice between stairways, ramps, escalators, and elevators exists. Further facility elements that need to be specified include horizontal walkways, waiting areas and platforms. For each element, its position and characteristic dimensions (such as the length for walkways) are to be determined.

Once the topology of pedestrian facilities is specified accordingly, a preliminary assessment of walking distances may be made, verifying that the required transfer times are met. This should include the specific needs of people with reduced mobility. Generally, the topology should allow for short and direct connections between facilities.

For the planned extension of Lausanne railway station, the topology of pedestrian facilities is changed in that it incorporates a newly-built metro station for local transit, it directly connects to a museum complex to the northwest of the station, and in that it features three instead of two transversal pedestrian underpasses.

IV. Demand Prediction. An estimate of pedestrian demand is required. Using the framework discussed in Section 5, it can be obtained based on the traffic concept of the train station and the topology of pedestrian facilities. In the estimation process, the impact of congestion on demand is usually neglected. In principle, it would be possible to take that influence into account. However, it requires a joint application of the demand estimator and the traffic assignment model, as well as a detailed layout of facility elements, which is not available at this point. It would thus require a merging of demand estimation and dimensioning (see step IV. below), which is cumbersome and rarely done in practice.

In the case of Lausanne, the influence of congestion is neglected in the

estimation of demand. To obtain a prediction for the year 2030, the planned instead of the current timetable is used, and the ridership is increased based on available forecasts.

V. Dimensioning. Based on the network topology, a detailed dimensioning of facility elements is to be made. In this process, a traffic assignment model as described in Section 6 is useful. It quantitatively predicts the level-of-service that results for a given demand estimate as a function of the facility layout.

Thanks to a joint consideration of pedestrian facilities, a consistent layout is reached in which the dimensions are balanced across elements. This is crucial for instance for cross-sections of adjacent facilities, such as the width of walkways and stairways. In a second step, the placement of travel services and furniture, such as ticket machines, information panels, benches or mobile sales points can be considered. As a general rule, these should not obstruct the main paths that connect facility elements.

In certain areas of railway stations that are only lightly loaded, a dimensioning based on the resulting level-of-service may be inappropriate. Instead, standard values for cross-sections should be used. Corresponding specifications are often provided by national authorities that seek a minimum degree of comfort (Buchmüller and Weidmann, 2008).

In the case of Lausanne railway station, the development of the network topology and the dimensioning has been a highly iterative process. Several times, the number of pedestrian underpasses has been changed from two to three and vice versa. In the beginning, our mandate consisted mainly in determining an appropriate width of these transversal underpasses. However, it turned out that the main bottleneck is rather the *connection* between these transversal underpasses and lateral platform access ways. By ‘smoothing’ a previously rectangular layout, the level-of-service can be increased more significantly than by simply enlarging the width of pedestrian underpasses. This finding was only possible due to the joint consideration of the underpasses and their platform access ramps, and due to a realistic estimation of demand that yields multi-directional flow. In the literature, a similar example of smoothing a rectangular bottleneck is discussed by Helbing et al. (2001), who use an evolutionary algorithm to improve the design of pedestrian facility elements.

VI. Verification. Evidence is to be provided that the dimensioning fulfills the specified functional requirements, including the desired level-of-service. Due to legal requirements, such a verification typically needs to be done manually. Significant differences in national legislations exist in how such a verification is performed.

In the case of Lausanne railway station, we have provided recommendations regarding the dimensioning of the main walking facilities. However, we have not been involved in the finalization of the layout, nor in its legal verification.

8 Concluding remarks

A framework for assessing the usage of rail access facilities has been discussed. It consists of a methodology for estimating pedestrian origin-destination demand based on the train timetable, and of a traffic assignment model for estimating the resulting level-of-service. The complete modeling framework is freely available (Hänseler et al., 2014b; Hänseler and Molyneaux, 2015).

Results from a case study of Lausanne railway station have shown that dynamic OD demand, level-of-service maps and travel time distributions can be accurately predicted. Required in that process are in particular the train timetable and ridership information.

The modeling framework has been embedded in a six-step planning process that is useful for practitioners and researchers confronted with the task of designing rail access facilities for a new train station to build, or an existing one to expand.

9 Acknowledgment

We thank Bilal Farooq, Nicholas Molyneaux, Marija Nikolić and Michaël Thémans for their contributions. The research presented in this report has been conducted in the framework of ‘PedFlux’, a three-year research collaboration between EPFL’s Transportation Center and SBB’s rail access division (I-AT-BZU). Financial support by SBB-CFF-FFS and the Swiss National Science Foundation (SNSF grant #200021-141099 ‘Pedestrian dynamics: flows and behavior’) is thankfully acknowledged.

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