

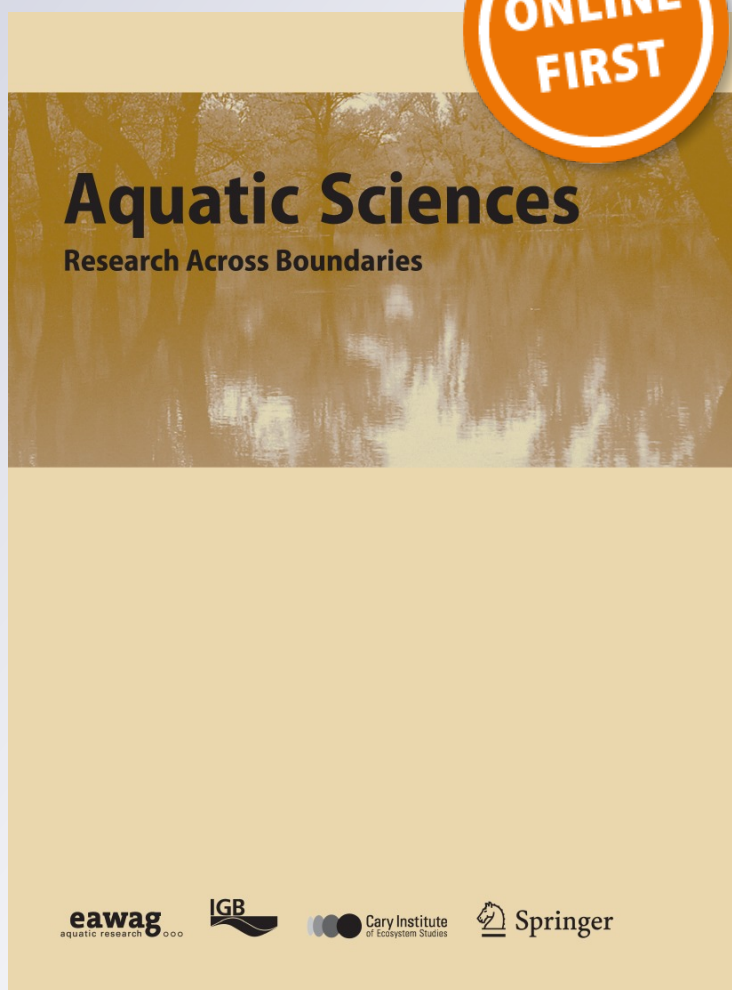
# *Attractiveness of a lateral shelter in a channel as a refuge for juvenile brown trout during hydropeaking*

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# Attractiveness of a lateral shelter in a channel as a refuge for juvenile brown trout during hydropeaking

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**Abstract** Peak power production in hydroelectric storage power plants results in frequent and intense flow variations in the rivers downstream of the plants. Fish populations can be negatively impacted when subjected to these so-called hydropeaking phenomena. In researching mitigation solutions, shelters in the riverbanks of channelized rivers have been identified as a means of protecting fish from excessive flow velocities. These shelters were studied systematically using juvenile brown trout (*Salmo trutta fario*) in an experimental configuration in which a straight channel was equipped with a lateral embayment. The purpose of the experiments was to generate hydrodynamic hydropeaking conditions in the channel that are undesirable for juvenile trout, thereby causing them to enter the shelter. The flow velocity distribution in the intersection plane between the main channel and the lateral shelter was found to be a significant parameter for attracting fish to the shelter. The utilization rate of trout in the shelter was used as a performance indicator. Using a basic rectangular shelter configuration without forced water exchange between the

shelter and the channel, the utilization rate was only 35 %. This rate was more than doubled by introducing a deviation groyne to force water exchange between the channel and the shelter. The position and orientation angle of this groyne were systematically varied to maximize the utilization rate. Maximum utilization rates approaching 90 % were obtained for an optimum configuration in which an island-type groyne was placed in the shelter. The results of the systematic channel tests showed the potential of the shelter to attract fish. Such a shelter could be used in channelized rivers both for morphological revitalization and to improve fish habitats. As a next step in this research, prototype shelters will be built on a natural river and monitored for 2–3 years under a hydropeaking flow regime.

**Keywords** Hydropeaking · Fish shelter · Groyne · Juvenile brown trout · Swimming trajectories · Ultrasonic doppler velocity profiler (UVP) · Velocity field

## Introduction

### Hydropower and hydropeaking in Switzerland

Hydropeaking is caused by increased electricity production at storage hydropower plants during high demand operations. Switzerland is one of the largest hydropower energy producers in the Alps in terms of its contribution to total electricity production (Schleiss 2007, 2012). Indeed, the peak energy production from storage power plants represents more than a third of the total electricity production (SFOE 2013). Thus, approximately 1,000 km of river lengths are affected by hydropeaking, especially the channelized Rhine and Rhône rivers, which have large

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reservoirs on tributaries located in alpine valleys (Meile et al. 2011a). Moreover, the Swiss Energy Strategy 2050 has determined that hydropower, especially peak energy production, will need to increase significantly until 2050 as a critical contribution for replacing existing nuclear power plants, reducing CO<sub>2</sub> emissions, and compensating for the highly fluctuating supply from renewables such as photovoltaic and wind energy. Thus, future hydropower production must focus even more strongly on peak energy generation. Consequently, hydropower production will affect the flow regime of rivers more severely, which will have to be assessed and mitigated using innovative measures. Many pre-Alpine and Alpine rivers have been implicated in this problem.

### Flow regime alteration by hydropeaking

Hydropeaking can strongly alter the hydrological flow regime of rivers (Meile et al. 2011a). In the absence of any precautions, the natural flow regime and associated hazards to rivers downstream of dams are generally replaced by the alternating, rhythmic and monotonous behavior of the outflow. Daily peak flows can reach up to 10–40 times the base flow, which generally corresponds to the natural flow or a residual flow. The negative impacts of such artificial flow regimes have been studied for over three decades (Baumann and Klaus 2003; Scruton et al. 2008). The ecological value of river reaches that are affected by hydropeaking can often be greatly reduced by significant changes in the river hydrological regime downstream of the restitution location of the turbinated water. The Fischnetz study (2004) showed that brown trout caught in Swiss rivers have diminished by approximately 60 % since 1980 (Peter and Schager 2004). This decrease has been attributed to hydropeaking and morphological alteration by channelization. The use of hydropeaking to change the flow regime is widely recognized as a major cause of disturbance to riverine ecosystems. Nuisances are often amplified by poor river morphology because of the channelization of the effected river sections. Thus, the 2011 Swiss Federal Law on Water Protection requires that owners of storage hydropower plants implement constructive measures to mitigate the negative effects of hydropeaking.

### Negative effects of hydropeaking and fish behavior

Hydropeaking is characterized by a sudden increase in flow velocities, resulting in a high peak discharge followed by a rapid decrease in discharge to a low value (Bruder et al. 2012). Sudden increases in flow velocities cause mortality in fish and invertebrates (Jungwirth et al. 2003). Less mobile macro-invertebrates and juvenile fish cannot swim sufficiently fast to find refuge in low

velocity areas in large flow recirculation zones (i.e., such as around boulders, scours, and roots) when present or in substrate interstitial spaces, and thus drift with the flow (Bruno et al. 2009). The risk of drift for aquatic organisms is strongly affected by the morphological condition of the river and the density of refuges (Young et al. 2011). Water is turbinated through the powerhouses directly from reservoirs; thus, in the winter, a rapid increase in discharge occurs in combination with a sudden increase in water temperature, which influences the behavior of invertebrates (Carolli et al. 2012; Zolezzi et al. 2011). Bruder et al. (2012) reported that brown trout have difficulty moving up in the Alpine Rhine in a high speed flow and that the fish try to reach shelters to conserve energy. During the peak discharge, mobilization of the riverbed sediment can damage exposed organisms and destroy habitats in the substrate interstitial spaces (Jones et al. 2011). The degradation of natural habitats has also been observed (Valentin 1996; Ovidio et al. 2008, Gouraud et al. 2008) for a bedload transport regime that was similarly highly modified (Baumann and Klaus 2003; Eberstaller and Pinka 2001). Turbidity also increases during peak flow, producing a high seepage gradient into riverine aquifers in an inner colmation of the river bed (Fette et al. 2007). When the turbines are closed, the rapid lowering of the water surface level can strand fish on the substrate of a high water riverbed (Baumann and Klaus 2003). Rivers with a natural morphology pose a greater danger of stranding aquatic organisms than channelized rivers (Young et al. 2011; Nagrodski et al. 2012). Tuhtan et al. (2012) documented for juvenile grayling that reaches with wider and flatter cross-sections posed higher stranding risks than reaches with steeply incised channels.

Hydropeaking negatively affects fish behavior (Heggnes et al. 1996; Valentin 1996; Taylor et al. 2012; Capra et al. 2012), fish migration (Greenberg et al. 1996), spawning habitats and egg development (Courret et al. 2012), as well as juvenile development (for ages 0+) (Scruton et al. 2005, 2008). Significant differences between modified and natural flowing rivers have been observed in terms of the abundance and distribution of some sensitive invertebrate taxa, fish diversity and the energy base of the food web (Smokorowski et al. 2011). Taylor and Cooke (2012) found that changes in flow regimes of a river can influence non-migratory fish behavior and impact habitat use and energy budgets. Korman and Campana (2009) found evidence that the growth of an age-0 rainbow trout improved on days with reduced hydropeaking, and individuals were found in immediate shoreline areas with higher water temperatures and lower velocities.

In hydropeaking rivers, the density and growth rate of juveniles was found to be reduced, and the mesohabitat was

disturbed (Jensen and Johnsen 1999; Flodmark et al. 2006; Korman and Campana 2009). Juveniles are most endangered by displacement or stranding because of their inability to find appropriate shelter, particularly in channelized rivers. Therefore, in the present study, we identify an optimum entrance design of lateral shelters that can be implemented in the riverbanks of channelized rivers to attract juvenile trout even under severe hydropeaking conditions. The lateral shelter may also serve as a refuge for other less mobile aquatic organisms, such as invertebrates.

### Mitigation of hydropeaking effects

Hydropeaking effects can be mitigated using operational, structural and morphological measures. Operational measures in storage power plants can result in severe economic consequences (Gostner et al. 2011) and are often not feasible. Structural measures include the construction of free-surface or underground compensation basins and bypass tunnels and channels (Bruder et al. 2012). Morphological measures resulting from river restoration projects can dampen the effect of flow variations. Meile et al. (2011b) investigated how introducing macro-roughness in riverbanks can dampen hydropeaking flow variations. The most efficient results can be achieved by routing the hydropeaking flow to compensation basins that serve as multipurpose reservoirs (Heller and Schleiss 2011). Person et al. (2014) provided a detailed overview on mitigation measures used to improve fish habitats in Alpine rivers that are affected by hydropower operations.

It can be counterproductive to try and improve river morphology within the framework of river restoration projects without preventing hydropeaking, which may increase the risk of stranding (Bruder et al. 2012). Under hydropeaking flow regimes, morphological measures must be designed to create habitats that remain stable during high discharge variability, i.e., without excessive flow velocities and dewatering. This challenge may be met by using specially designed lateral refuges in riverbanks of channelized rivers that serve both as fish shelters and stable habitats for small aquatic organisms. Fish shelters are not new and have been commonly used to mitigate the effects of high flow velocities (Scruton et al. 2008). Valentin (1995) and Korman and Campana (2009) highlighted the significance of using lateral bank refuges to protect fish and other aquatic organisms from rapid variations in hydraulic parameters and to provide better growth conditions. Motivated by the results of the aforementioned studies, systematic channel experiments were used in this study to investigate the attraction of juvenile brown trout to a lateral shelter as a function of its entrance geometry.

## Materials and methods

### Goals and research conditions

The ultimate goal of this study was to formulate criteria for the entrance design of shelters that can be implemented at riverbanks in channelized alpine rivers with a hydropeaking regime. The scientific objective was to understand and subsequently influence the behavior of fish that are subjected to excessively high velocities in the main river, such that these fish can be directed toward velocity refuges in the banks. The research methodology was based on tests on wild fish using an experimental flume. The flume was used to simulate hydrodynamic flow conditions that are hostile to fish and as they occur during hydropeaking in a channelized river downstream of the water restitution of powerhouses of storage hydropower schemes. The objective of this approach was to develop and optimize an entrance design for use as a shelter that can attract fish under hydropeaking conditions. The utilization rate of fish in the refuge was used as a performance indicator of the test configurations.

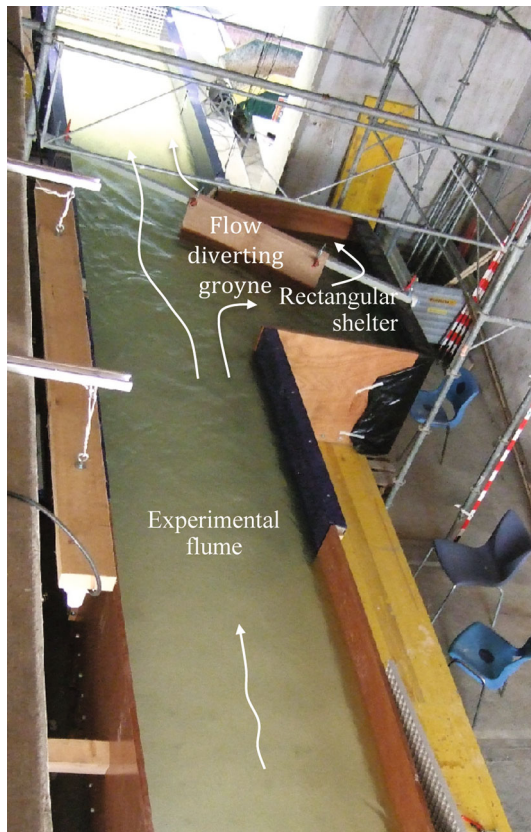
### Experimental study

#### *Experimental configuration and test refuge entrance configurations*

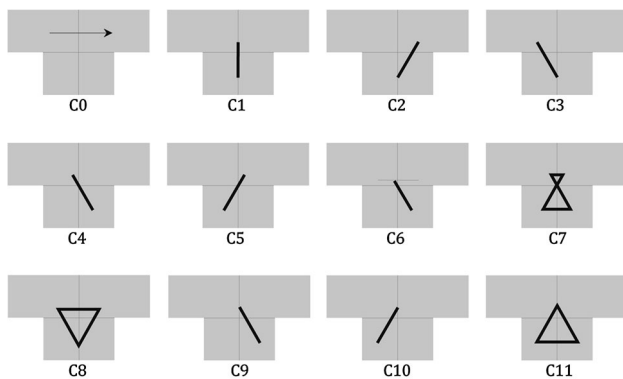
To identify the optimum shelter configurations for attracting fish, fish were exposed to hydropeaking sequences in a channel equipped with a lateral refuge. A special flume was built in the old powerhouse of the Maigrage dam in Fribourg (Switzerland) to obtain a permanent supply of fresh water from the reservoir (Fig. 1) and control the light intensity. The channel had an effective length of 12 m and a width of 1.2 m. The refuge area had a 2-m length and a 1.2-m width and was located on the right bank.

A total of 12 configurations with different water diverting structures at the refuge entrance were tested, as shown in Fig. 2, and the geometries of these structures are detailed in Table 1. To activate the flow exchange between the channel and the refuge, a simple vertical wall was first inserted into the refuge over the entire water depth to serve as a diverting structure, as shown by configuration C1 in Fig. 2. The outer edge of the diverting wall protruded 0.30 m into the channel section. The inner edge was located 0.50 m from the refuge sidewall. Except for configuration C6, these values were maintained throughout all of the tested geometries by changing the angle of the wall to the flow direction (Fig. 2). Finally, with an eye toward practical applications, diverting structures with island-type geometries were also tested (see Fig. 2: C7, C8, C11).





**Fig. 1** Downstream *topview* of ecohydraulic test flume installed in the former powerhouse of the Maigrauge dam in Switzerland



**Fig. 2** Tested configurations for different shelter entrances: the *bold line* represents the structures tested for diverting water through the shelter

The bottom of the channel and the refuge were covered with a mixture that was two-thirds rounded medium gravel, ranging from 16 to 32 mm in diameter, and one-third coarser gravel, ranging from 30 to 60 mm in diameter. This composition corresponded to a substrate preference curve that has been reported by Valentin (1996) and Vismara et al. (2001) for juvenile brown trout. Preliminary tests on

this loose substrate showed that during hydropeaking some juveniles hid in the spaces between the coarse gravel that were near the channel wall where velocities were somewhat lower. The juveniles remained almost immobile during the entire experiment even after hydropeaking subsided, showing signs of physical stress by moving their gills rapidly. To simulate fairly hostile conditions in the channel, the space between the coarse gravel was filled with mortar and a 25-cm wide rough concrete plate (with a washed gravel surface) was placed along the channel wall. The corner between the channel bed and sidewall was filled with a 10-cm chamfer with a 45° angle. Such hostile conditions can be found in channelized rivers that are subjected to hydropeaking. In rivers below hydroelectric projects, it is very common for an armor layer of cobbles to form that can become embedded by fine sediments. This type of bed serves as a very poor velocity refuge for fish during hydropeaking, as was found using the experimental channel. Nevertheless, a loose substrate was maintained in the refuge to simulate favorable natural conditions. The coarse gravel bed was painted white throughout the channel and the refuge to enhance the visibility of fish for camera tracking.

#### *Conditions for fish experiments*

Hydropeaking conditions were produced by suddenly opening the gate for regulating the water supply. Discharge, water depths and water temperature were continuously measured during the tests.

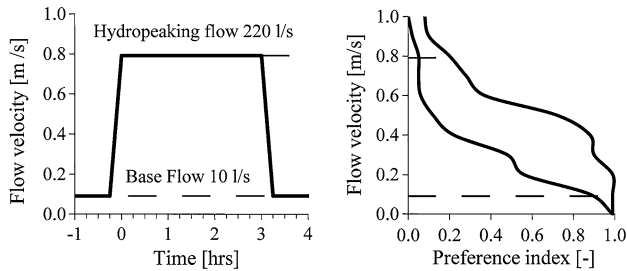
All of the tests were performed for a hydropeaking event in the flume, starting from a base flow of 10 l/s that corresponded to a 10-cm water depth in the flume. Then, the discharge was rapidly increased over 15 min to a maximum peak discharge of 220 l/s, which resulted in a water depth of 24 cm in the flume. The evolution of the corresponding flow velocities in the flume is shown in Fig. 3 (left), which is compared directly with the velocity preference curve for juvenile brown trout (*Salmo trutta fario*) (i.e., ages of 0+ and 1+) from Vismara et al. (2001). The base discharge of 10 l/s for mean flow velocities of approximately 0.1 m/s occurred in the flume, resulting in a favorable preference index above 0.9 (see Fig. 3, right). Flow velocities rapidly increased over 15 min to approximately 0.8 m/s (corresponding to an increase of 0.05 m/s per min), resulting in very hostile conditions with a preference index below 0.2. These velocities are beyond the sustained swimming ability of juvenile brown trout.

Tests were performed with wild juvenile brown trout (*Salmo trutta fario*) (i.e., ages of 0+ and 1+), which were captured by electrofishing in a small stream in the Swiss Midland (at Tannenbach Buttisholz near Lucerne) prior to testing each configuration. Table 2 details the

**Table 1** Geometric parameters obtained from Fig. 2 for the tested refuge configurations: (1) length of the interface section upstream and downstream of the flow diverting structure, (2) upstream and

downstream orientation angle of the flow diverting structure, and (3) depth of protrusion of the flow diverting structure into the section of the main channel

Configuration	C0	C1	C2	C3	C4	C5	C6	C7	C8	C9	C10	C11
Upstream length (m)	–	0.99	1.38	0.60	0.99	0.99	0.99	0.99	0.60	1.17	0.81	0.81
Downstream length (m)	–	0.99	0.60	1.38	0.99	0.99	0.99	0.99	0.60	0.81	1.17	0.81
Upstream angle (°)	–	90	120	60	60	120	60	120	60	60	120	120
Downstream angle (°)	–	90	120	60	60	120	60	60	120	60	120	60
Protrusion depth (m)	–	0.30	0.30	0.30	0.30	0.30	0.15	0.30	0.30	0.30	0.30	0.30



**Fig. 3** Channel hydraulic parameters related to preference index for juvenile brown trout, taken from the results of different studies by Vismara et al. (2001)

**Table 2** Characteristic (length) of fish captured by electrofishing in the Tannenbach river at Buttisholz village near Lucerne, Switzerland

Date of electric fishing	08.08.08	14.10.08	15.05.09	05.10.09
Number of fish caught	21	22	33	20
Average length (mm)	165	164	125	151
Maximum length (mm)	196	196	161	187
Minimum length (mm)	139	139	88	107
Standard deviation (mm)	19	17	18	18

characteristics of the fish that were captured by electrofishing. Brown trout is the main species found in Alpine and sub-alpine rivers and has been subject of many biological research studies on hydropeaking (Valentin 1996; Scruton et al. 2003; Flodmark et al. 2006; Gouraud et al. 2008; Murchie et al. 2008). During the experiments, groups of 10 (low density) or 20 trout (high density) were used, which resulted in a fish density of 1–2 fish/m<sup>2</sup> in the channel and 10–20 fish/m<sup>2</sup> in the refuge. These low densities correspond to densities that have been observed in rivers with an average habitat quality for juvenile brown trout; the high densities correspond to trout streams with an excellent habitat quality for juvenile fish (Arrignon 1998; Schager et al. 2007). During hydropeaking, the density of the fish in the refuge increased and reached relatively high values over a small area. Brown trout often accumulate in high densities in a refuge. The velocities and cover provided by the refuge enable the juvenile fish to successfully

maintain their position and feed on drifting macro-invertebrates during hydropeaking events. The ability to feed and grow in these refuge habitats is a significant benefit for these fish, especially if hydropeaking events occur on a frequent (e.g., daily) basis. However, macro-invertebrates were not used in the tests that were conducted in our artificial channel.

Before performing a test, a uniform flow of 10 l/s was established in the flume. The fish were then introduced in the channel entrance in a fenced repose area where they could acclimate to the water conditions. The fence was then removed, and the flow in the channel was increased from 10 to 220 l/s in approximately 15 min and maintained at its maximum value for 3 h, as shown in Fig. 3. The positions of the individual fish were visually recorded every 20 min during the hydropeaking period, and the density of fish for the tested refuge entrance configuration was plotted (Ribi 2011).

A video camera was placed perpendicularly above the refuge to track the fish. The video recordings were analyzed frame-by-frame for particularly interesting configurations to identify preferential pathways.

Each refuge configuration was tested three times and always involved a newly captured group of fish (see Table 3): first, two different groups, A and B, of 10 fish were used, followed by a combined group, A&B, of 20 fish. Six series of 20 fish were used for 36 experimental sequences, corresponding to a total of 12 tested entrance configurations (Fig. 2). That is, a group of 10 fish was used only twice to test the same entrance configuration of the shelter and for two to a maximum of four configurations. Typically, each fish was used for a maximum of six hydropeaking events. No change in fish behavior or learning effects was observed during the tests. Table 3 details the electro-fished groups that were used for each test and each configuration.

To prevent weight loss in the trout from bad conditions, the trout were fed before each test and three times per week with macro-invertebrates that were captured in a river near the powerhouse. The fish were allowed to recover for at least 36 h between each test (Table 3). Salmonid fish generally recover rapidly from stress and swimming performances (Wedemeyer and Wydoski 2008).

**Table 3** Detailed test program showing the electro-fished fish group used for each configuration, where italicized dates indicate the first time the fish group was submitted to a test

Date of electric fishing	Configuration	Group A	Group B	Group A&B
08.08.08	C0	<i>11.09.08</i>	<i>10.09.08</i>	12.09.08
08.08.08	C1	15.09.08	16.09.08	18.09.08
14.10.08	C2	<i>20.10.08</i>	<i>23.10.08</i>	27.10.08
14.10.08	C3	28.10.08	03.11.08	10.11.08
14.10.08	C4	04.11.08	06.11.08	07.11.08
15.05.09	C5	<i>20.05.09</i>	<i>22.05.09</i>	25.05.09
15.05.09	C6	28.05.09	28.05.09	02.06.09
15.05.09	C7	04.06.09	04.06.09	05.06.09
05.10.09	C8	21.10.09	21.10.09	19.10.09
05.10.09	C9	<i>08.10.09</i>	<i>08.10.09</i>	12.10.09
05.10.09	C10	16.10.09	16.10.09	14.10.09
05.10.09	C11	23.10.09	23.10.09	26.10.09

The tests were performed over 3-h sequences in the spring and autumn when the water temperatures ranged between 6 and 14 °C. Figure 4 shows the temperature that was continuously and automatically measured during the tests. From the end of May to the beginning of June 2009, the automatic measurement system was out of service: thus, the mean water temperature of approximately 10 °C was measured manually. These temperatures are optimum for trout (Elliott 1994; Küttel et al. 2002; Jungwirth et al. 2003). In preliminary tests that were performed with significantly colder water, the fish exhibited a weak response to the refuge and did not enter it.

#### Numerical flow field simulation

The different entrance configurations of the shelters were compared using a numerical analysis of the local velocity distributions. The primary focus was on the flow exchange between the channel and the shelter and the velocity pattern at its entrance intersection. A systematic analysis was performed using a 2D simulation model based on shallow water equations. BASEMENT “BASic EnvironMENT for natural flow and hazard simulation” (Faeh et al. 2010) was used for these purposes. The model was used to solve the unsteady flow equations at an average depth using the finite volumes numerical pattern. SMS, i.e., the “Surface Water Modeling System” was used in parallel to build the simulation grid, to pre- and post-process the data and to illustrate the results. Figure 5 shows an example of such a simulation in which the base configuration C0 without a diverting structure was compared with the C8 configuration in which a triangular island was used as a diverting structure.

#### Flow field measurements

Horizontal flow velocities were measured at four different flow depths by an ultrasonic Doppler velocity profiler (UVP) (Met-Flow 2002) using 6 transducers at 1 MHz that were mounted on a measurement frame. Figure 6 shows the measurement transects that were obtained using the six transducers. The vertical entrance interfaces between the refuge and the channel were investigated in detail along with two sections across the channel that were 2 m upstream and 0.8 m downstream from the shelter. Conventional measurements were also performed locally using a micro current-meter for validation. The vertical profiles were recorded six times at each measurement section at 0.025, 0.05, 0.075 and 0.1 m (near the water surface) above the bottom. The measured velocities have been presented in detail in Ribí et al. (2010) and Ribí (2011). Particular attention was focused on measuring the velocity at a depth of 0.025 m above the bottom, which corresponded approximately to the swimming depth of fish approaching and entering the shelter (Ribí 2011).

#### Results

##### Velocity distributions at the interface between the channel and the refuge

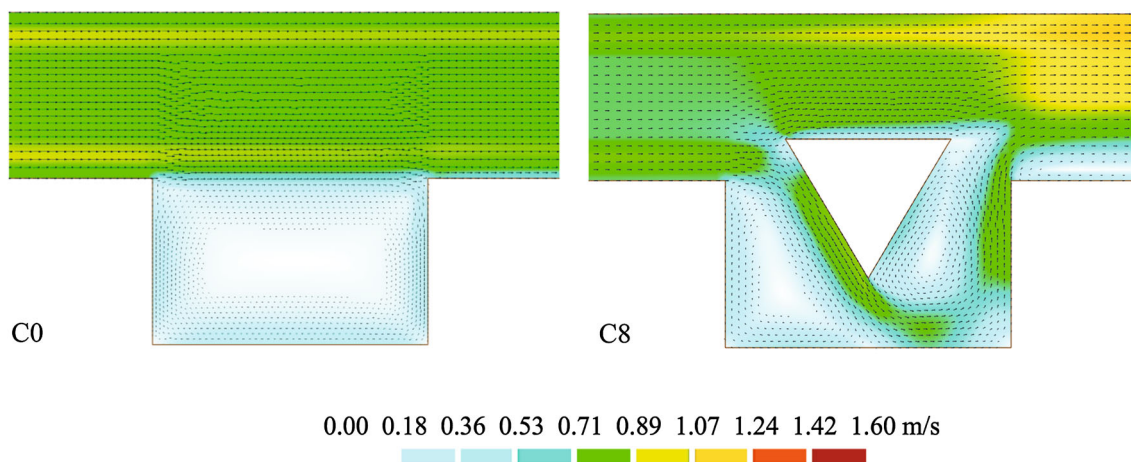
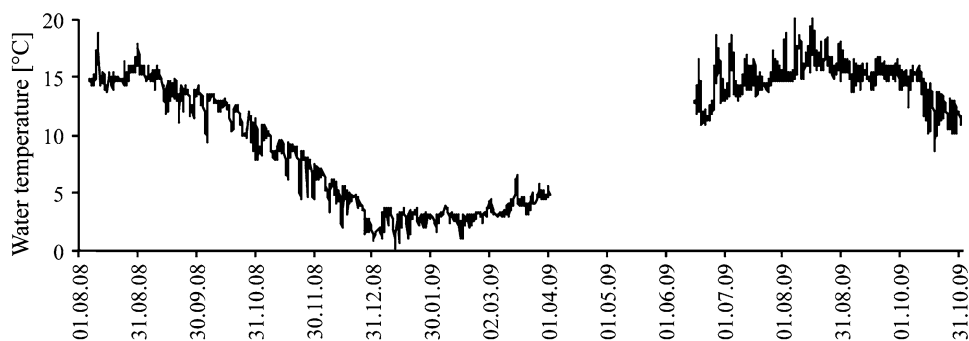
Figure 7 is a plot of the measured horizontal velocity distributions at the interface between the channel and the shelter for all configurations with water diverting structures (C1–C11). Each distribution along the interface transect of 2 m is presented for 4 different water depths, as previously mentioned. Negative velocities indicate that the water was flowing from the channel into the refuge, and positive velocities indicate the converse. Configuration C0 is not presented because the zero exchange velocities were measured along the interface. In general, for all configurations, the water diverting structure forced a significant amount of water to enter the refuge upstream. However, the water left the refuge downstream of the diverting structure. In some of the tested configurations, some quantity of water left and entered at the same time upstream and downstream of the diversion structure: this behavior is most clearly visible for C2, C9, C10 and C11. This behavior indicates the presence of shear zones with zero flow velocities.

##### Fish utilization rates in the refuge

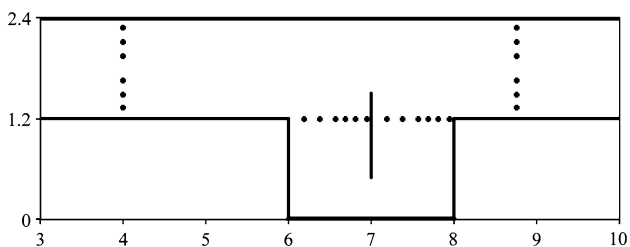
During each test, the number of fish in the shelter was counted every 20 min. Figure 8 shows the test results as a



**Fig. 4** Water temperature of the Sarine River at the powerhouse of Maigrange dam: the broken lines indicate the testing periods



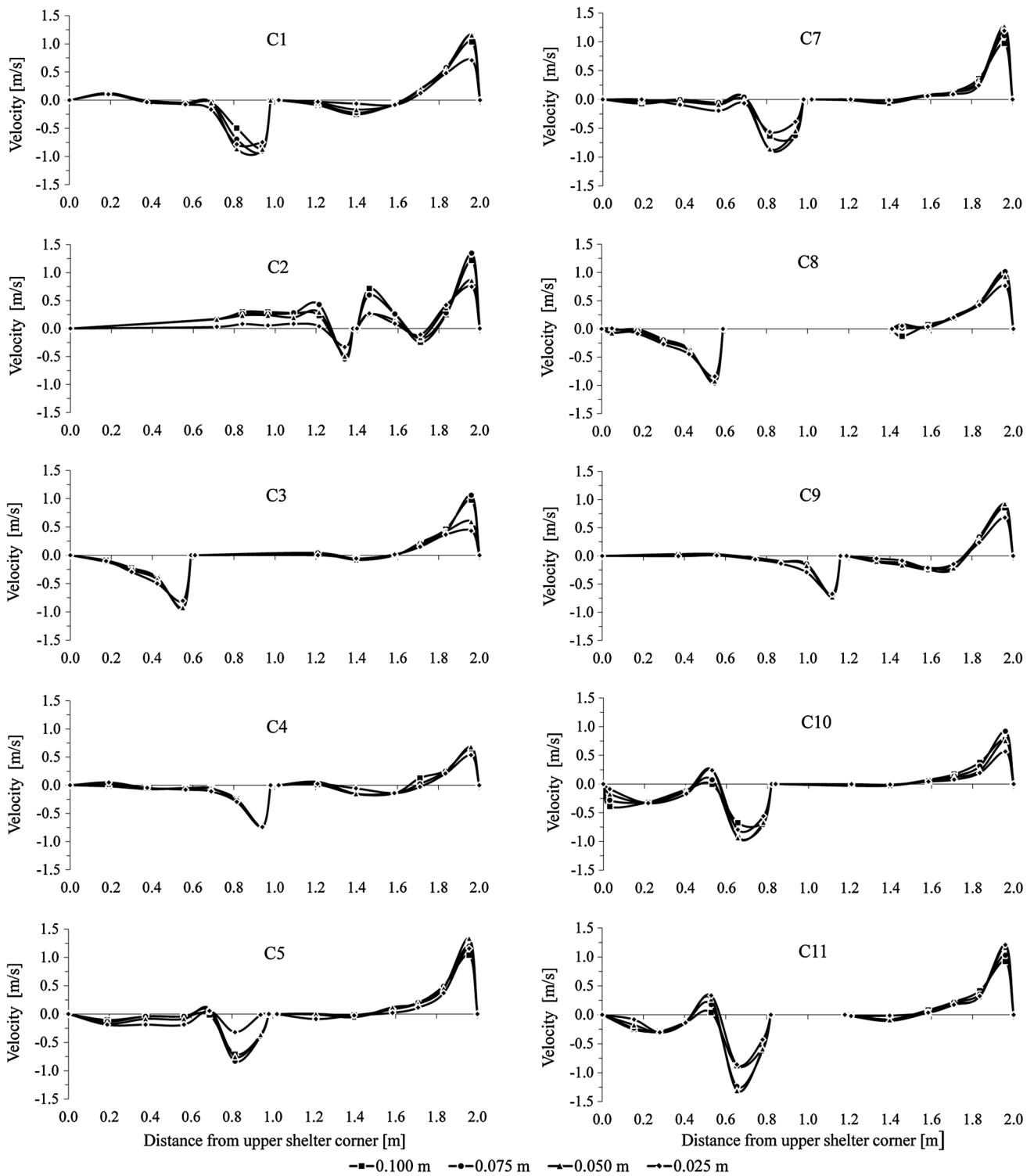
**Fig. 5** Flow velocity fields simulated using BASEMENT 2D for configurations C0 and C8: the water flows from *left to right*, and the average flow velocity in the main channel is 0.79 m/s



**Fig. 6** Six UVP transducers distributed across **a** the interface section between the refuge and the channel and **b** the channel sections upstream and downstream of the shelter

percentage of the total number of fish that were used (10 or 20) for configurations C0 and C8. A fairly strong variation in fish presence rate in the refuge over 3 h of investigation can be observed (Fig. 8). However, averaging the three tests every 20 min (see the bold line in Fig. 8) revealed a clear trend in the presence rate for the different configurations. To further analyze the capacity of the different entrance configurations of the shelters to attract fish, fish utilization rates in the refuge were defined by time

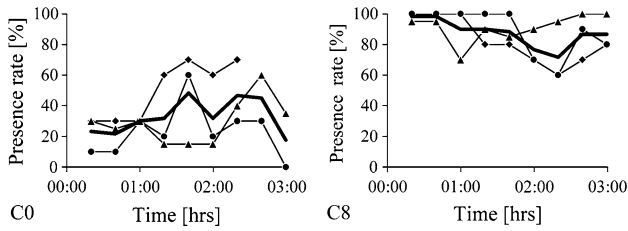
averaging the aforementioned averaged presence over the total duration of the experiment (3 h). The results are compared in Fig. 9 for all of the configurations, yielding boxplots for the time averaged utilization over 3 h of testing, the first and third quartiles and the extreme values. The simple lateral cavity without a diverting structure (C0) exerted a very weak attraction on the fish at an average utilization of the refuge of approximately 33 % (Fig. 9: C0). This low attraction could be attributed to the non-existent flow exchange between the main channel and the refuge. Therefore, the transit or diverted flow across the refuge was computed by integrating the simulated and measured velocities over the vertical plane separating the refuge from the main channel. Figure 10 shows the results as absolute values of the diverted discharge and as relative values in terms of a percentage of the total discharge in the main channel. For the C0 configuration without a diverting structure, no water was exchanged between the main channel and the refuge. As soon as a water diverting structure was inserted at the shelter entrance, the water exchange increased considerably, ranging from 11 % (C6) to 22 % (C3).



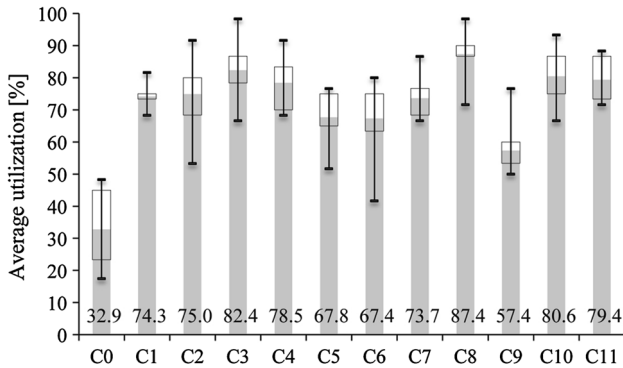
**Fig. 7** Measured horizontal velocity distributions at the interface between the channel and the shelter at 0.025, 0.05, 0.075 and 0.1 m (near the water surface) above the channel *bottom*

Figure 11 compares the average utilization rate to the percentage of the discharge that was diverted from the main channel into the refuge. For all configurations with a water diverting structure, the average utilization rates

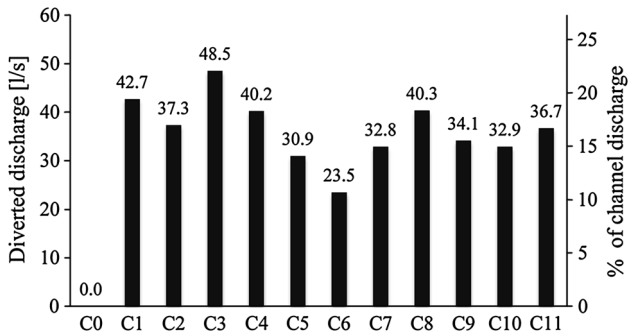
ranged from 57.4 (C9) to 87.4 % (C8), whereas the diverted flow rate ranged between 23.5 and 48.5 l/s, respectively, which was 11 to 22 % of the total discharge in the experimental channel. Averaging all configurations,



**Fig. 8** Presence rate of fish in the refuge, which was counted during three tests that were conducted every 20 min for configurations C0 and C8: the *bold line* represents the average of the three tests

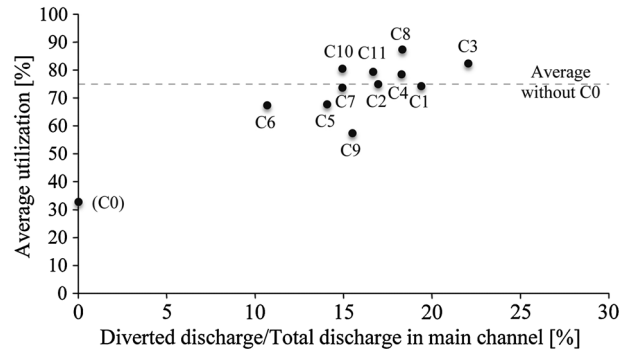


**Fig. 9** Boxplots of the fish utilization rate in the shelter, which is time-averaged over three tests for each configuration to yield the average value over a 3-h test, the first and third quartiles and the maximum and minimum (all of the values are averages of the three tests for each configuration from Fig. 7)



**Fig. 10** Absolute and relative values of the diverted discharge from the channel through the refuge for all configurations: the relative values are given as a percentage of the total flow of 220 l/s in the channel

but excluding C0, yielded a utilization rate of approximately 75 % (Fig. 11). For the configurations considered, the utilization rate was above 60 % as soon as approximately 15 % of the water was diverted from the channel into the refuge.



**Fig. 11** Average utilization rate of the refuge by the fish as a function of the relative diverted discharge from the channel into the refuge: configuration C0 is shown for reference

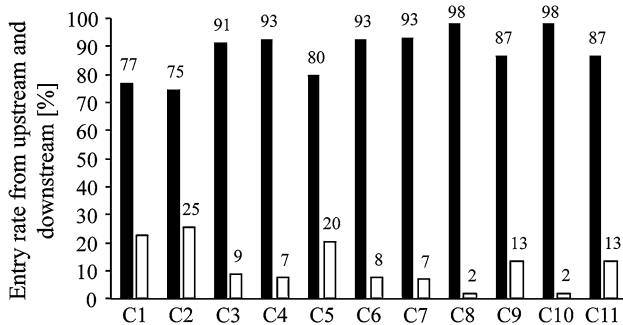
### Location of fish entries into the refuge

A video recording of the preferred travel path of fish from the channel into the refuge over a 3-h hydropeaking event was used to perform a detailed analysis of each entrance configuration. The results are summarized in Table 4. Figure 12 also shows the number of fish that entered from upstream or downstream of the refuge for all of the configurations with water diverting structures. The number of entries per fish is also indicated in Table 4. Figure 12 shows that the fish entering the shelter during hydropeaking followed a clear preferential travel path from downstream. Figure 13 shows that the fish found a path upward of the channel along the right sidewall leading to the downstream corner of the refuge to take advantage of the relatively low velocities in that region. Individual fish rested for a few seconds as they reached the low velocity area before crossing into the higher velocity flow issuing from the refuge edge to reach the shelter behind the water diverting structure. The fish then temporarily rested behind the water diverting structure before entering deeper into the refuge, as shown in Fig. 13.

Table 4 shows the calculated statistical number of entries per fish. Less than 1 entry was generally observed for entries from upstream, whereas entries from downstream ranged from 2 to 6. However, an analysis of the fish motion showed that there was no direct relationship between the number of entries and the utilization rate. Observations during the hydropeaking event showed that fish traveled continuously between the refuge and the channel. Some configurations elicited more fish movement than others. For example, the C2, C5, C6, C8 and C9 configurations were characterized by less than 3 entries per fish per test, i.e., the fish stayed longer in the refuge. The C3, C4, C7, C10 and C11 configurations produced more than five entries per fish per test, indicating that the fish were leaving and returning to the refuge more frequently

**Table 4** Preferential entries of fish from the channel into the refuge for all of the tested configurations: (1) fish presence rate in the refuge, (2) diverted discharge into the refuge, and (3) length of the interface section upstream and downstream of the flow-diverting structure (see also Table 1)

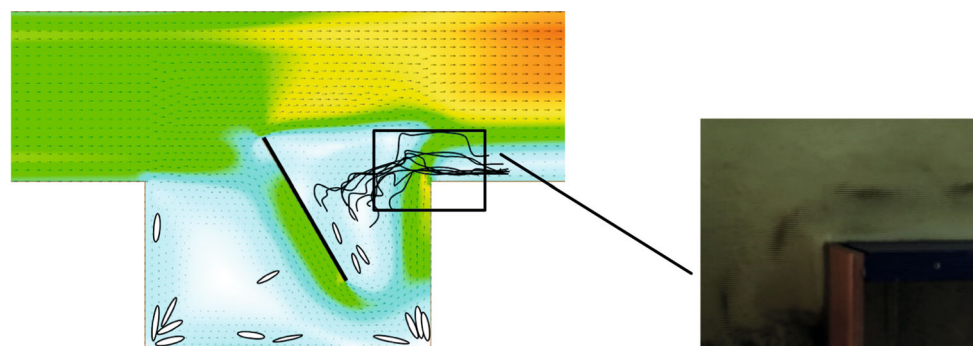
Configuration	C0	C1	C2	C3	C4	C5	C6	C7	C8	C9	C10	C11
Presence rate (%)	32.9	74.3	75.0	82.4	78.5	67.8	67.4	73.7	87.4	57.4	80.6	79.4
Diverted discharge l/s	0.0	42.7	37.3	48.5	40.2	30.9	23.5	32.8	40.3	34.1	32.9	36.7
Upstream length (m)	–	0.99	1.38	0.60	0.99	0.99	0.99	0.99	0.60	1.17	0.81	0.81
Downstream length (m)	–	0.99	0.60	1.38	0.99	0.99	0.99	0.99	0.60	0.81	1.17	0.81
Number of fish	20	21	22	11	11	20	20	10	20	19	20	15
Number of upstream entries		19	13	5	4	10	3	5	1	6	2	13
Number of downstream entries		64	38	52	50	39	37	68	54	39	101	84
Total number of entries	38	83	51	57	54	49	40	73	55	45	103	97
Downstream entry rate (%)		77	75	91	93	80	93	93	98	87	98	87
Number of downstream entries per fish		0.90	0.59	0.45	0.36	0.50	0.15	0.50	0.05	0.32	0.10	0.87
Number of upstream entries per fish		3.05	1.73	4.73	4.55	1.95	1.85	6.80	2.70	2.05	5.05	5.60
Total number of entries per fish	1.90	3.95	2.32	5.18	4.91	2.45	2.00	7.30	2.75	2.37	5.15	6.47



**Fig. 12** Entry rate of fish into the refuge near the interface sections that are located downstream (black bars) and upstream (white bars) of the water diverting structure

than for the previously mentioned configurations. The C4 configuration elicited between three and five entries per fish per test. The greatest movement was observed for the C7 configuration with more than seven entries per fish per test. The fish remained longer in some shelter configurations; however, the fish only used these shelters intermittently for recovery and not as a permanent abode during hydropeaking.

**Fig. 13** Example of video image processing for configuration C4, showing trajectories of juvenile trout entering the refuge from downstream and their position in the refuge: comparison with the velocity field simulated using 2D BASEMENT



Finally, the video recordings were used to count the number of fish entries in the refuge for each configuration along the interface section that was divided into 0.1-m intervals (see Table 5). These values were combined with the velocities at a 0.025-m depth above the bottom within the same interval to determine the preferred fish velocities, as is shown in Fig. 14 for the C4 and C8 configurations. The highest number of entries occurred in the shear layers between the inflowing and outflowing water, where the most frequent velocities ranged between 0 and 0.2 m/s, reaching values up to 0.3 m/s. The distribution of the preferred velocities for the entry of the fish into the refuge corresponded to the distribution that has been observed for juvenile brown trout in rivers (Souchon et al. 1989; Vismara et al. 2001; Ayllón et al. 2009).

## Discussion

The fish could easily find a refuge during hydropeaking when a certain amount of water was diverted from the



**Table 5** Number of fish passages entering the shelter, counted in intervals of 10 cm along the interface section between the shelter and the channel for each test configuration: the distance is measured from the upstream corner of the shelter; the bold line indicates the position of the diverting structure

Configuration	C0	C1	C2	C3	C4	C5	C6	C7	C8	C9	C10	C11
Distance (m)												
0.00					Upper corner of shelter							
0.05	0	0	0	0	0	0	0	0	0	0	0	0
0.15	2	1	0	3	0	0	2	0	0	2	0	1
0.25	0	3	0	2	1	1	0	0	1	0	0	3
0.35	2	1	1	0	0	2	0	2	0	1	1	4
0.45	2	8	1	0	2	0	0	2	0	1	1	1
0.55	3	3	0	0	0	2	1	1	0	1	0	1
0.65	1	2	1	0	1	2	0	0		0	0	3
0.75	1	0	0	1	0	2	0	0		1	0	0
0.85	0	0	2	0	0	0	0	0		0	0	
0.95	3	1	2	5	0	1	0	0		0	0	
1.05	5	1	5	2	1	0	0	0		0	0	
1.15	3	0	1	2	1	2	1	0		0	0	
1.25	2	0	0	5	4	4	0	2		0	1	5
1.35	0	2	0	7	3	5	3	14		0	4	19
1.45	1	5	6	7	2	6	2	17	4	0	8	12
1.55	1	7	9	6	8	3	6	11	18	3	7	10
1.65	2	14	3	10	15	4	7	9	9	6	18	13
1.75	8	16	7	4	11	8	10	6	10	13	20	17
1.85	3	14	10	3	5	6	6	6	13	14	34	8
1.95	1	5	3	0	0	1	2	3	0	3	9	0
2.00					Lower corner of shelter							

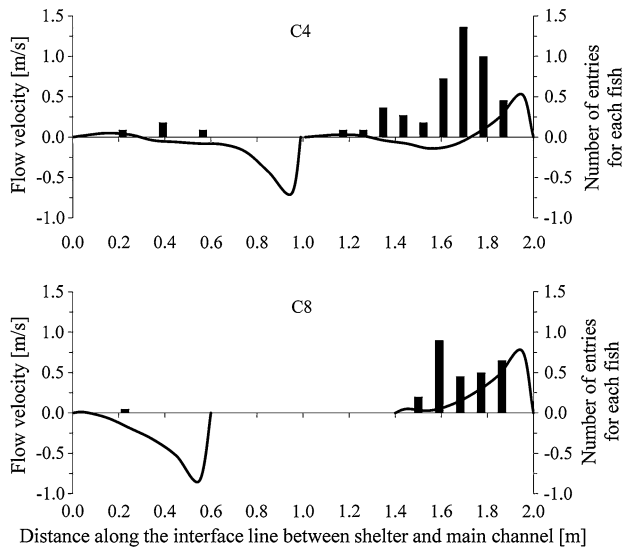
channel into the refuge. A simple diverting wall was used for the C1 configuration, which significantly increased the utilization rate of the shelter to 74 % during hydropeaking in the tests (Fig. 9) for a diverted discharge of 43 l/s (Fig. 10), which corresponded to 19 % of the channel discharge. No significant trend was observed for the relative attraction of the fish to the tested configurations for discharges that were diverted by more than 15 %. However, on average, a utilization rate in the 75 % range was observed for all of the tested water diverting structures (Fig. 11). The fish primarily entered the refuge from downstream for most of the configurations with an entry rate that was above 85 % (Fig. 12). In some configurations (C5 and C2), more than 20 % of the fish also entered from upstream. However, these configurations did not exhibit the best overall utilization rate (Fig. 9). The fish entered from downstream in the most attractive configurations, C3, C8 and C10, with utilization rates above 80 %. As previously mentioned, the highest number of entries occurred in the

shear layers between the water flowing into and out of the refuge, where the most frequent velocities 2.5 cm above the bottom ranged between 0 and 0.2 m/s, with values up to 0.3 m/s. However, the large number of fish entries cannot be explained only in terms of such low velocities. The fish appeared to be attracted to low velocities in the shear flow zones where water entered and left the refuge at the same time. This flow structure involving velocity shear zones appeared to be highly significant for creating an attractive refuge. However, the water jet that was issued at the lower edge of the shelter back into the channel, which is shown in Fig. 5 for configuration 8, was also very important in enabling the fish to find the shelter.

An entrance configuration for a refuge should be selected based on the utilization rate as well as the feasibility of the configuration in terms of its structural stability and integration into the river. As previously mentioned, configurations C3, C8 and C10 had the highest utilization rates (>80 %). The C3 and C10 configurations, in which

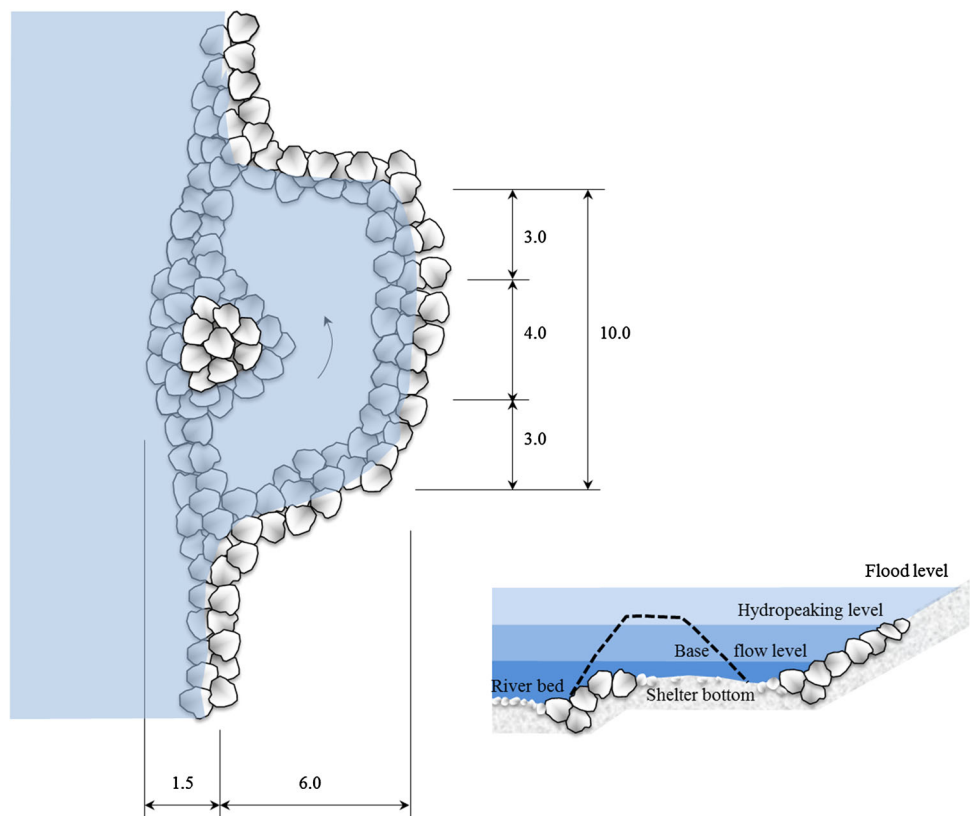
simple walls were used as water diverting structures, are not recommended because these configurations poorly satisfied the feasibility criterion for prototypes. In a prototype refuge, a thin wall would need to be simulated using

a groyne. The required protrusion into the main channel would produce scouring, endangering the stability of the head of the groyne. Thus, the C8 and C11 configurations, which are characterized by a deflecting structure such as a triangular island, could be considered to be more favorable in terms of stability and erosion resistance, if properly constructed. C8 should be preferred over C11. C8 exhibited the highest utilization rate (87 %), and the arrangement of the island, which pointed toward the interior of the refuge, occupied less space in the shelter, thereby creating more attractive flow conditions for the habitat of small aquatic organisms. Moreover, the velocity fields obtained using 2D numerical simulations showed that the derived flow followed a more clearly defined path and that the rotation cells were larger for C8 than in the other configurations. The small number of entries for the C8 configuration (which exhibited an average of 2.7 entries per fish per test) led us to conclude that this configuration provided more stable conditions that favored the presence of fish in the refuge. In configuration C11, fish movement into and out of the refuge was characterized by a high average number of entries of 6.5 per fish per test, indicating that the fish were less comfortable in this refuge. Finally, the contraction of the flow in the main channel in front of the island was less marked than for the C8 configuration. Therefore, the C8 configuration is recommended as a reference for experimental prototype shelters.



**Fig. 14** Distribution of flow velocities along the interface line (solid line) between the channel and the refuge at 0.025 m above the bottom, superimposed on the distribution of the number of entries per fish (vertical bars) and per 0.1-m interval for configurations C4 and C8

**Fig. 15** Sketches of the proposed refuge for entrance configuration C8, showing the minimum dimensions [m]: **a** top view and **b** cross-section



For practical applications, it is important to reproduce the same flow structure and the diverted discharge from the main channel into the refuge as observed in the experiments. The horizontal velocity distribution at the interface between the main channel and the refuge, as illustrated in Fig. 14, was a critical feature of the flow structure. Figure 15 shows that the banks of the refuge and the triangular island were designed to produce a similar diverted flow pattern as that observed in the experiments. Preliminary tests using driftwood in a physical experiment showed that openings of at least 3 m should be used on both sides of the island to prevent the shelter from being clogged by accumulated driftwood. Thus, the minimum length of a refuge should be on the order of 10–15 m. However, a small accumulation of driftwood and fine sediments in the water recirculation zones of the refuge could even be advantageous for creating additional shelter and habitats for aquatic organisms. To prevent diversion of bedload transport from the main channel into the refuge, the bottom of the refuge should be approximately 1 m above the riverbed. In addition, during low water conditions, a minimum water depth of at least 0.5 m should always cover the bottom of the refuge to avoid any stranding of aquatic organisms. The flow velocities through the refuge are sufficiently high to prevent significant deposition of fine suspended sediment, except in the water recirculation and calm water zones, depending on the geometry chosen in practice. As previously mentioned, the local deposition of fine sediments, such as sand and silt, can be considered to be favorable for the habitat potential of the refuge. Riparian vegetation at the refuge banks may also be an important issue in practical applications.

The favorable flow-through conditions that were observed for the C8 configuration lead us to expect that this configuration can also improve both the entrainment and circulation of food organisms (drifting macro-invertebrates) in the refuge area. A refuge habitat that provides foraging opportunities may be very important for fish growth, especially when hydropeaking events occur frequently or for long periods of time. The refuge could also provide an optimal level of visual cover for juvenile trout combined with using the aforementioned bank vegetation as overhead cover, which would allow greater numbers of these territorial fish to occupy the refuge area.

## Conclusions

Different entrance configurations of fish shelters were installed laterally in a channel for experimental tests on juvenile brown trout. Even under severe hydropeaking conditions, i.e., a sudden increase of 22 times base discharge, juvenile brown trout could rapidly find the shelter as long as the water exchange between shelter and channel

was sufficiently high to attract fish. This optimum behavior was obtained when approximately 20 % of the main channel flow was diverted into the lateral shelter. These conditions were obtained by placing a water diverting structure using a wall and island shapes in the middle of the embayment that slightly protruded into the main channel. The ability of the shelter to attract fish was analyzed by testing different orientations and protrusions of the diverting structure. Favorable conditions for attracting fish corresponded to water entering and leaving the shelter at the same time upstream and downstream of the diverting structure. When the fish headed into the shelter, the preferred fish flow path was in the shear zone between the flow entering and leaving the shelter, where the velocities were nearly zero.

Most of the fish found the shelter from downstream by swimming along the bank up to the jet leaving the refuge. After passing this jet, the fish easily entered the shelter along the aforementioned shear zone at very low flow velocities. When coming from upstream, the fish were also guided by the flow that was diverted into the shelter.

Systematic tests showed that a water diverting structure in the shape of a triangular island exhibited strong potential for attracting trout into the shelter. The tested refuge could only be considered to be a mitigation measure for hydropeaking in channelized rivers. On this basis, this research study will be followed up by implementing and monitoring prototype experimental shelters in a channelized river in a hydropeaking regime. Thus, the performance of this solution will be tested continuously in the natural environment using a variety of fish species at different stages of growth. Furthermore, the habitat potential for other aquatic organisms than fish will also be assessed by in situ monitoring.

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